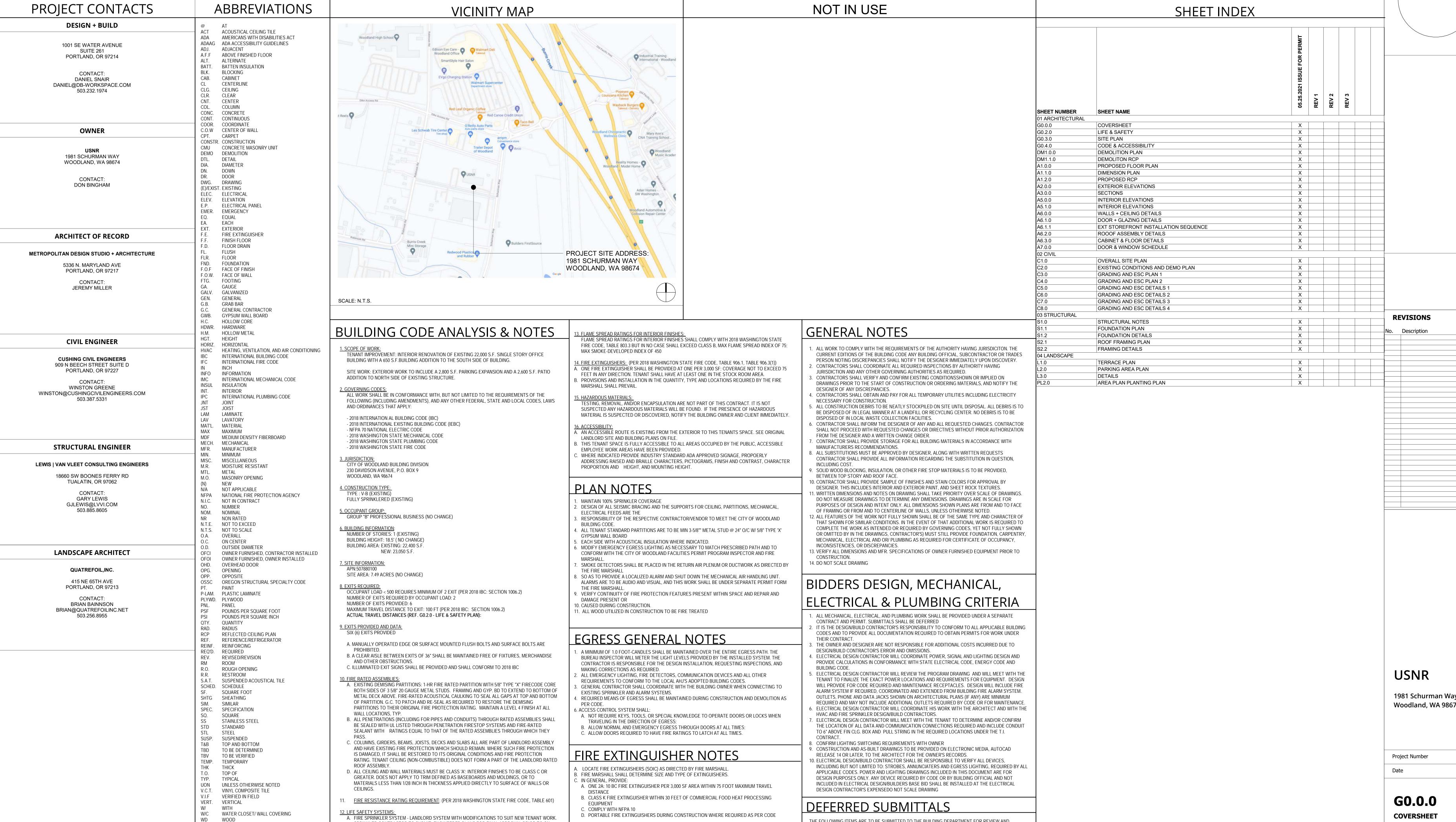
USNR

1981 Schurman Way Woodland, WA 98674



SPRINKLER CONTRACTOR TO SUBMIT ENGINEERED PLANS FOR FINAL APPROVAL PRIOR TO ON-

B. FIRE ALARM SYSTEM - LANDLORD SYSTEM WITH MODIFICATIONS TO SUIT NEW TENANT WORK. FIRE

ALARM CONTRACTOR TO SUBMIT ENGINEERED PLANS FOR FINAL APPROVAL PRIOR TO ON-SITE

EXIT ACCESS

TO EXIT DISCHARGE IS 300'-0".

FULLY SPRINKLERED TYPE B OCCUPANCY IN ACCORDANCE WITH SECTION 903.3.1.1 ALLOW COMMON

PATH TRAVEL DISTANCE OF 100'-0" TO EXIT ACCESS, MAXIMUM TRAVEL DISTANCE FOR TENANT SPACE

SITE WORK. CONTRACTOR TO PERFORM SITE VISIT TO VERIFY EXISTING CONDITIONS.

WORK. CONTRACTOR TO PERFORM SITE VISIT TO VERIFY EXISTING CONDITIONS.

C. EMERGENCY LIGHTING - NEW, PROVIDED BY TENANT

WIDE FLANGE

WITHOUT

WATERPROOF

WATER HEATER

THE FOLLOWING ITEMS ARE TO BE SUBMITTED TO THE BUILDING DEPARTMENT FOR REVIEW AND

APPROVAL PRIOR TO INSTALLATION:

1. HVAC

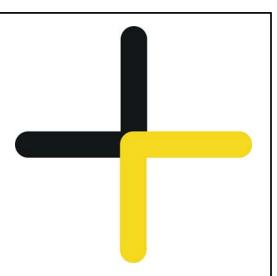
2. ELECTRICAL

3. FIRE SPRINKLER

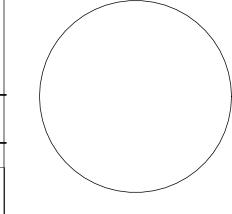
4. LOW VOLTAGE

STOREFRONT

FIRE ALARM 7. PLUMBING

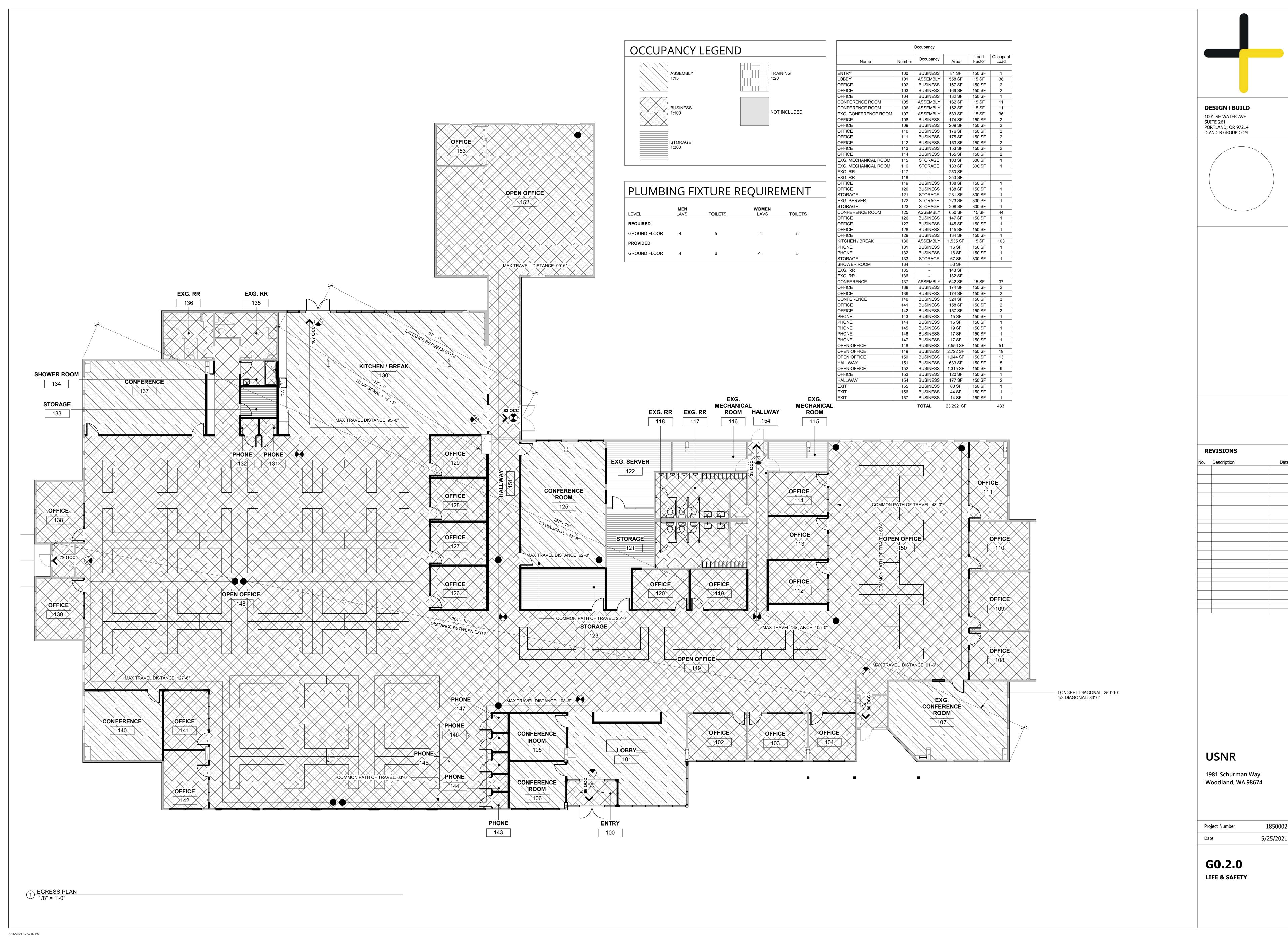


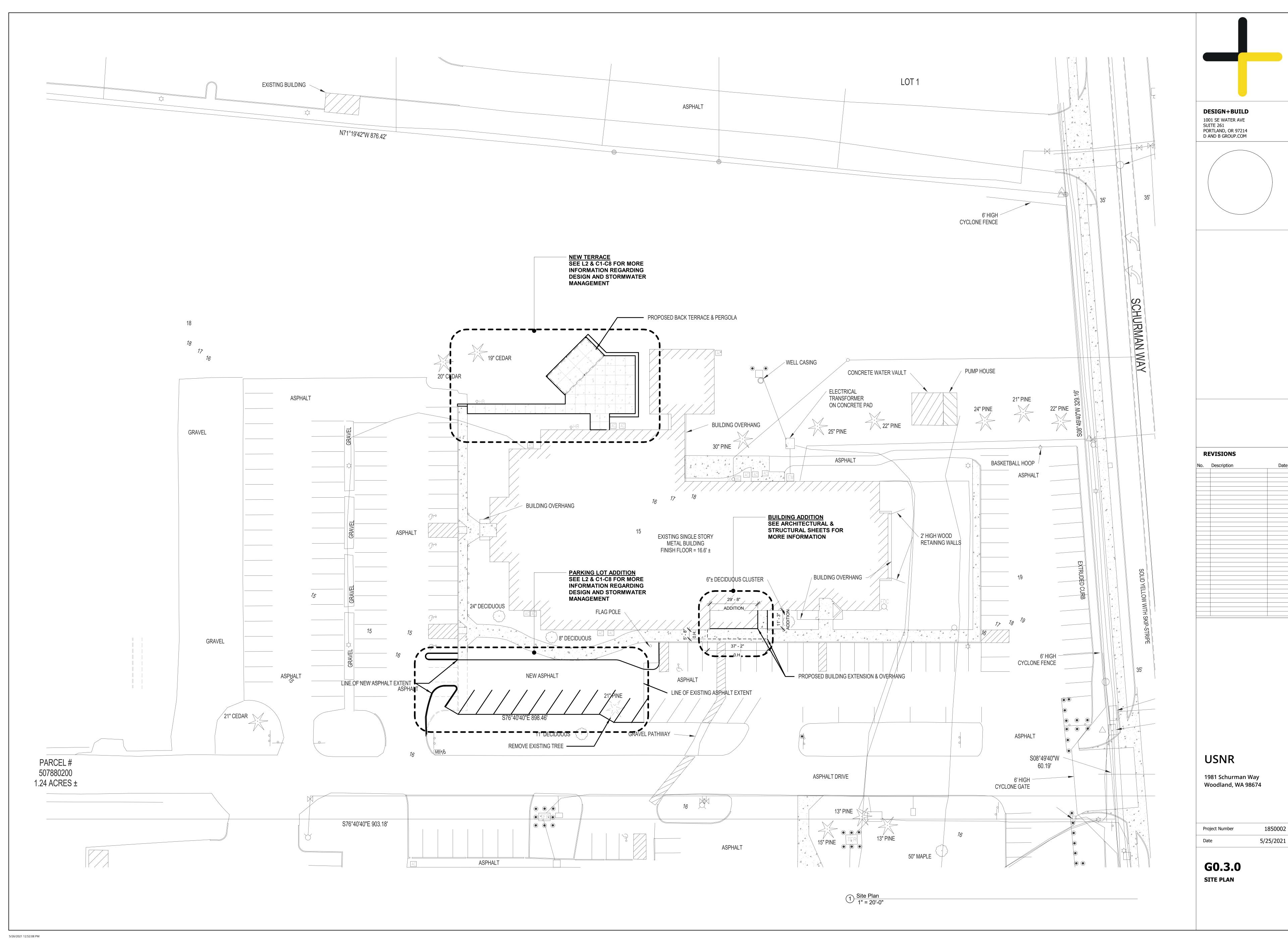
DESIGN+BUILD 1001 SE WATER AVE SUITE 261 PORTLAND, OR 97214 D AND B GROUP.COM

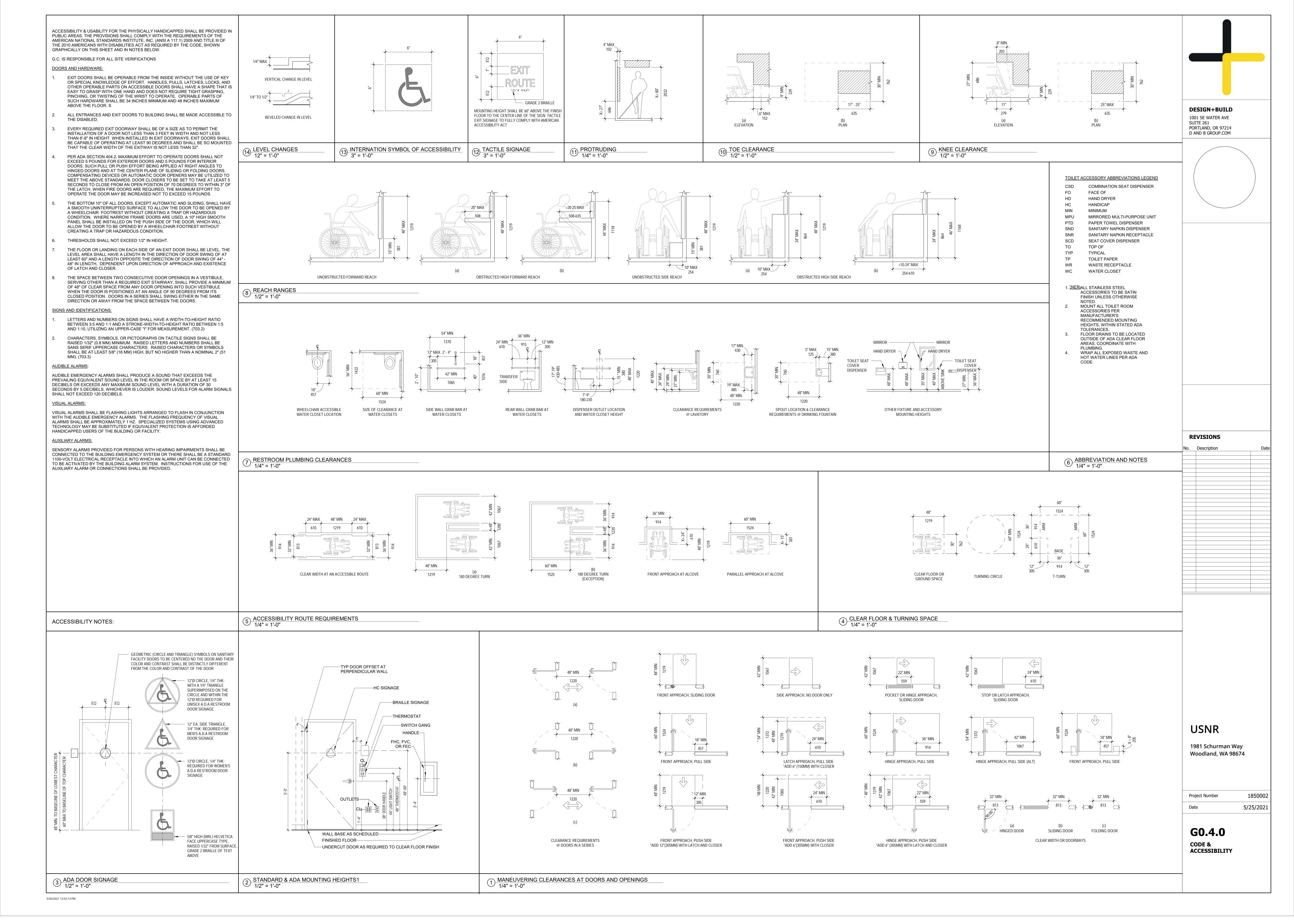


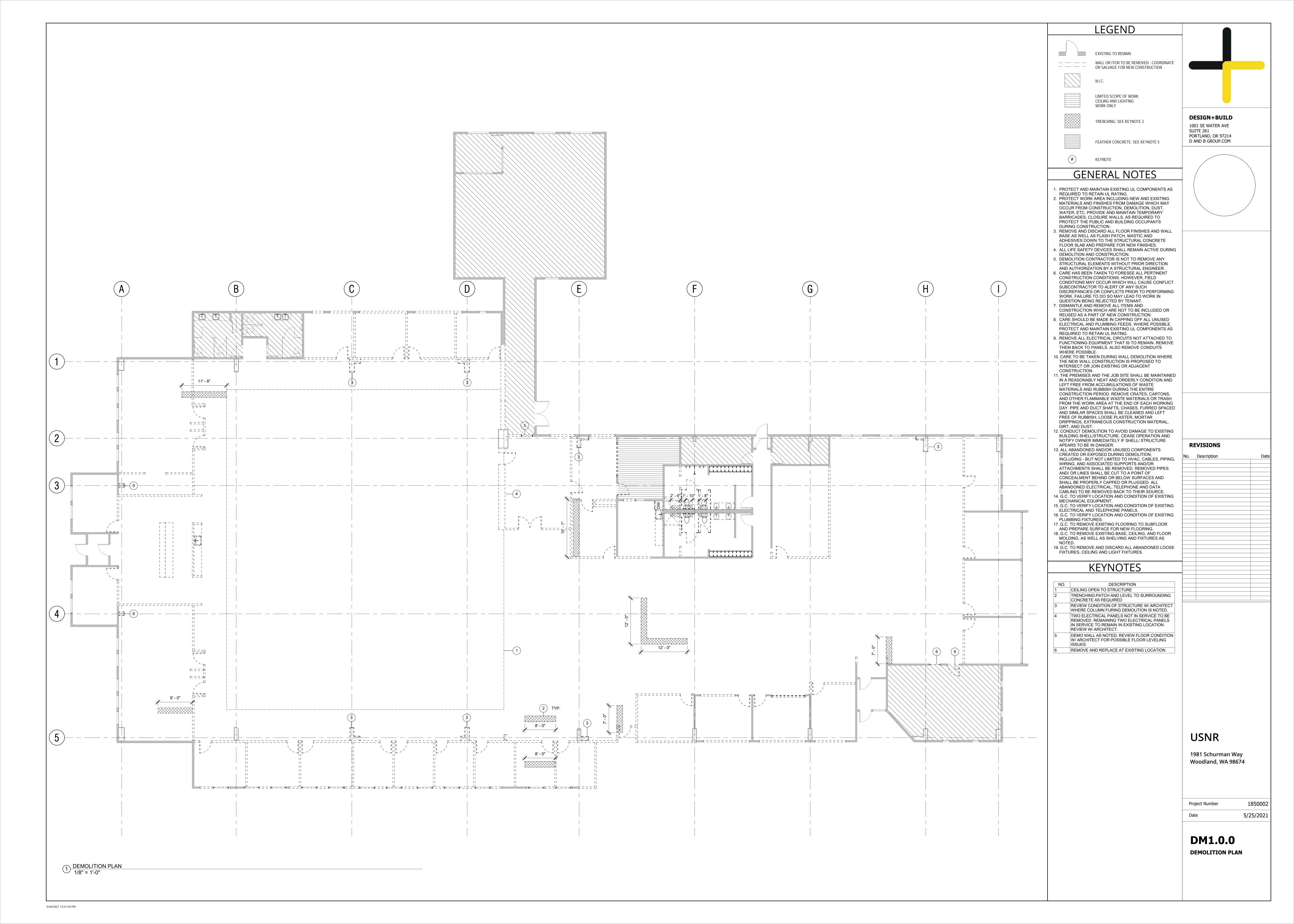
Woodland, WA 98674

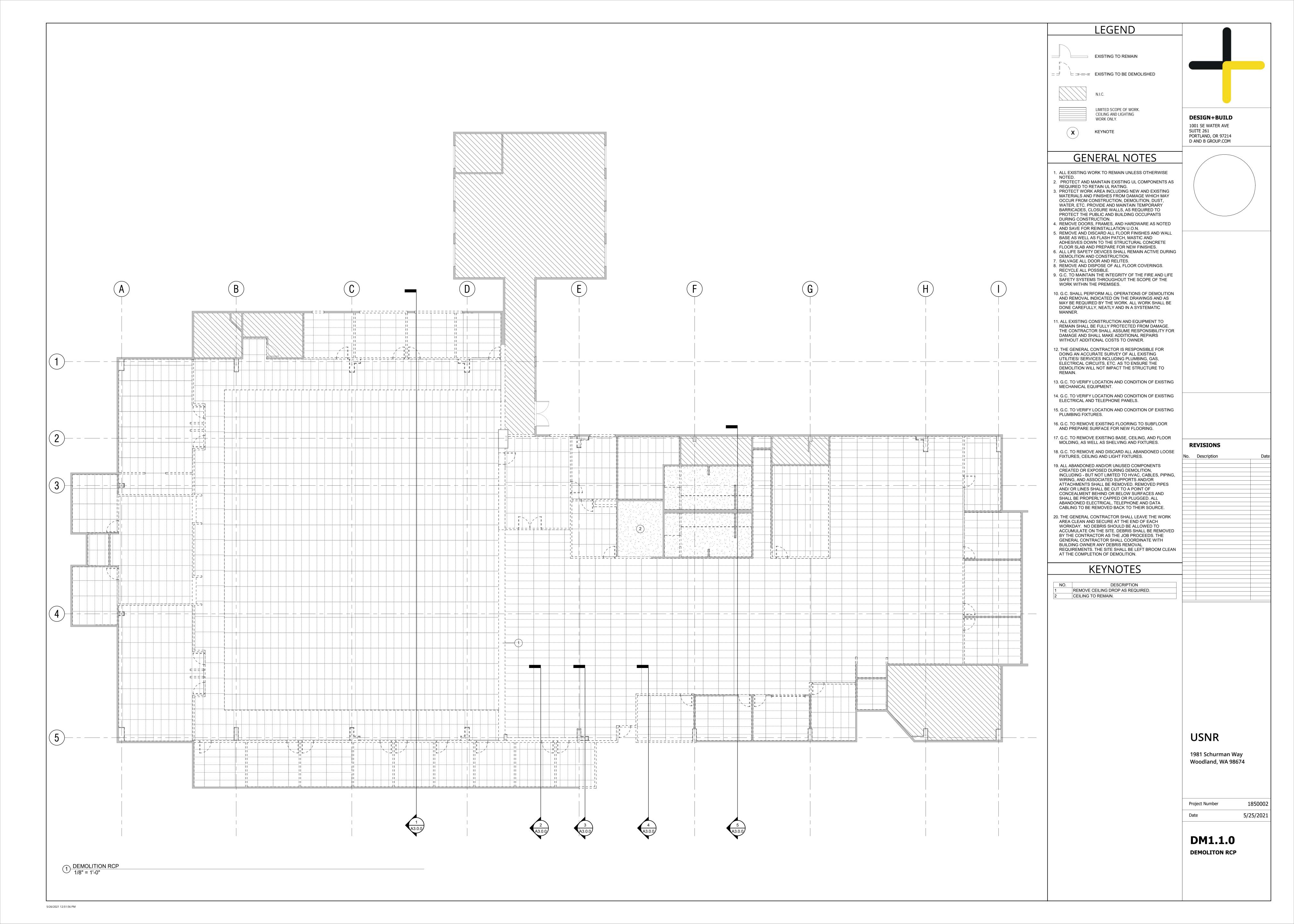
1850002

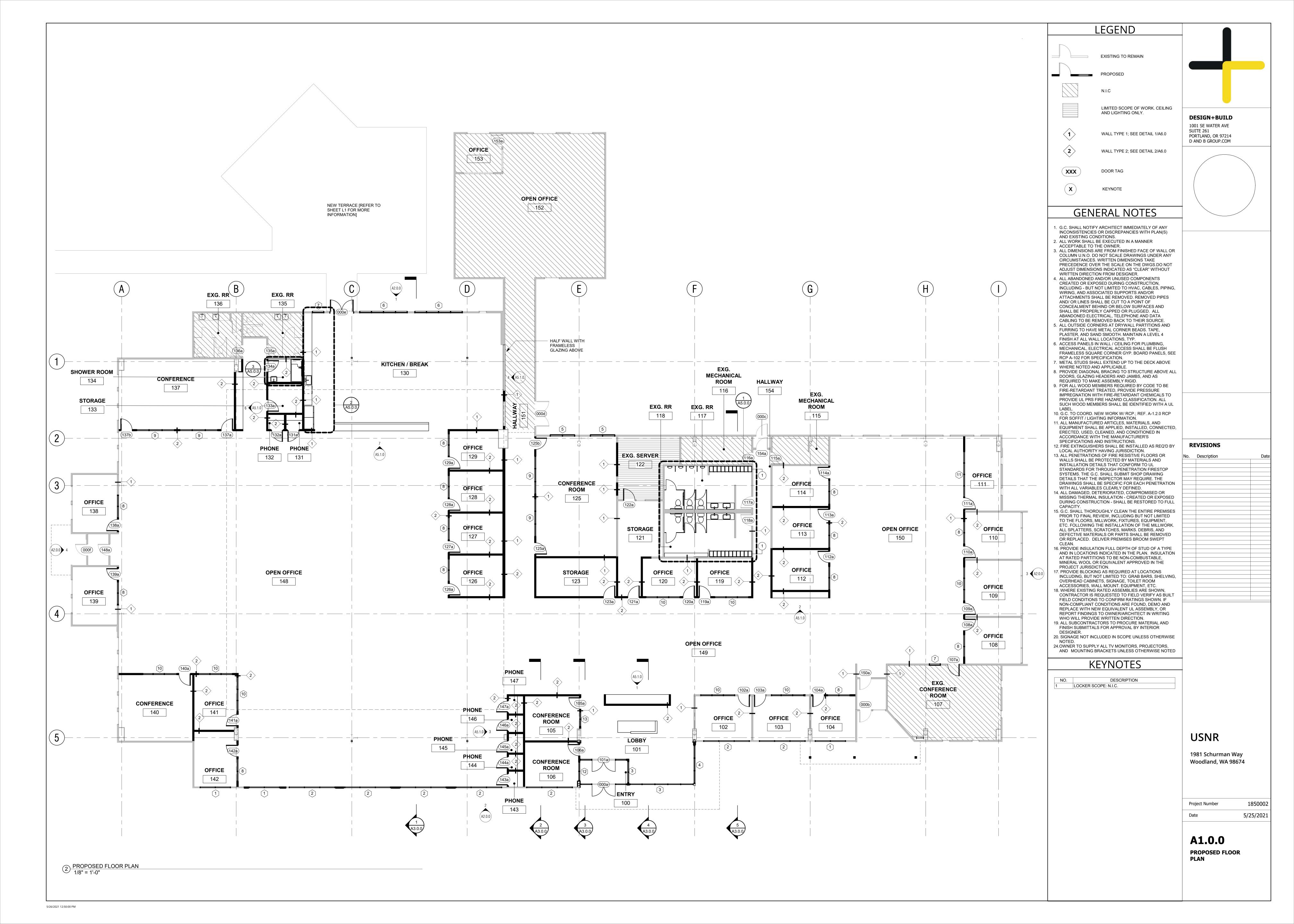


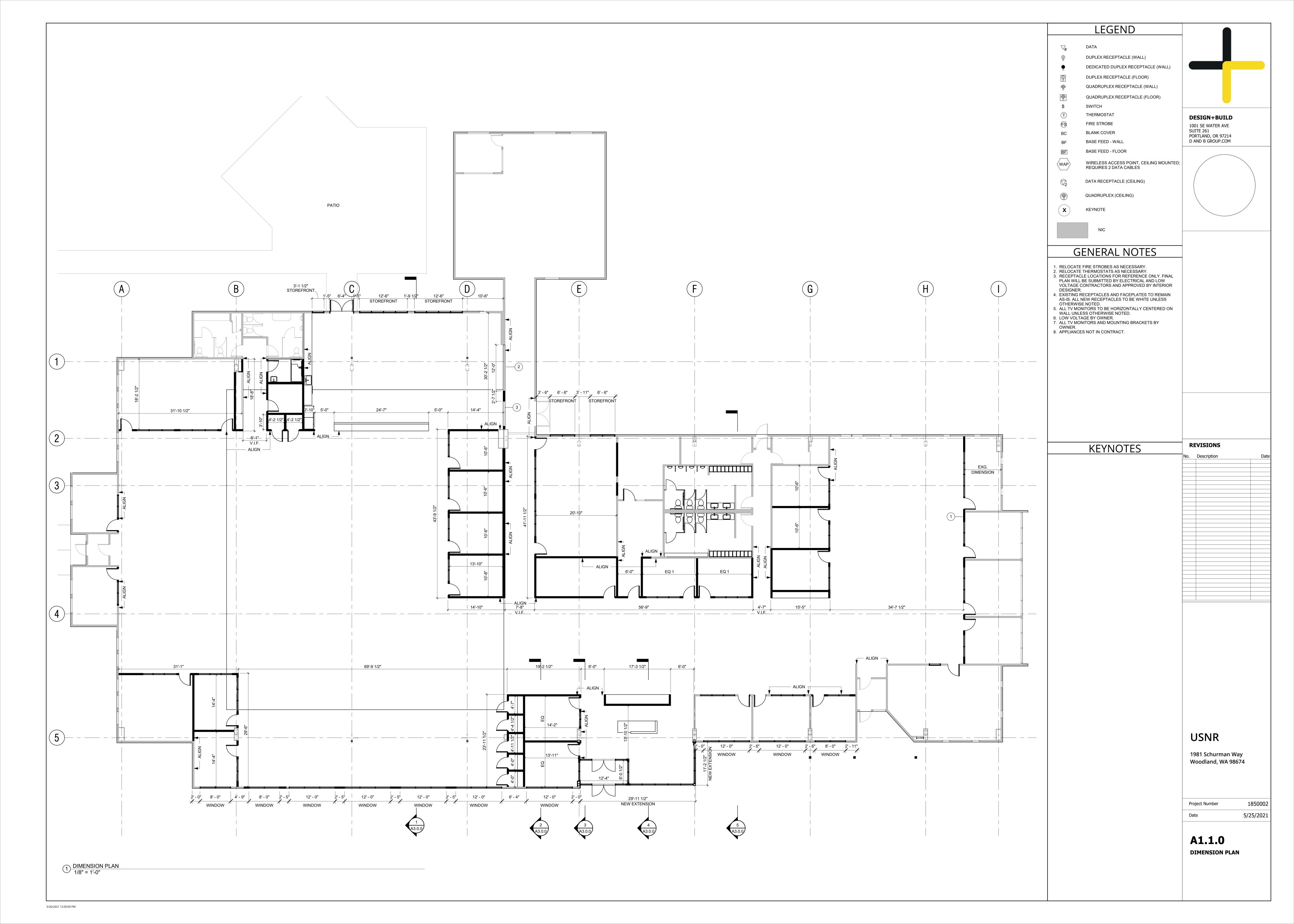


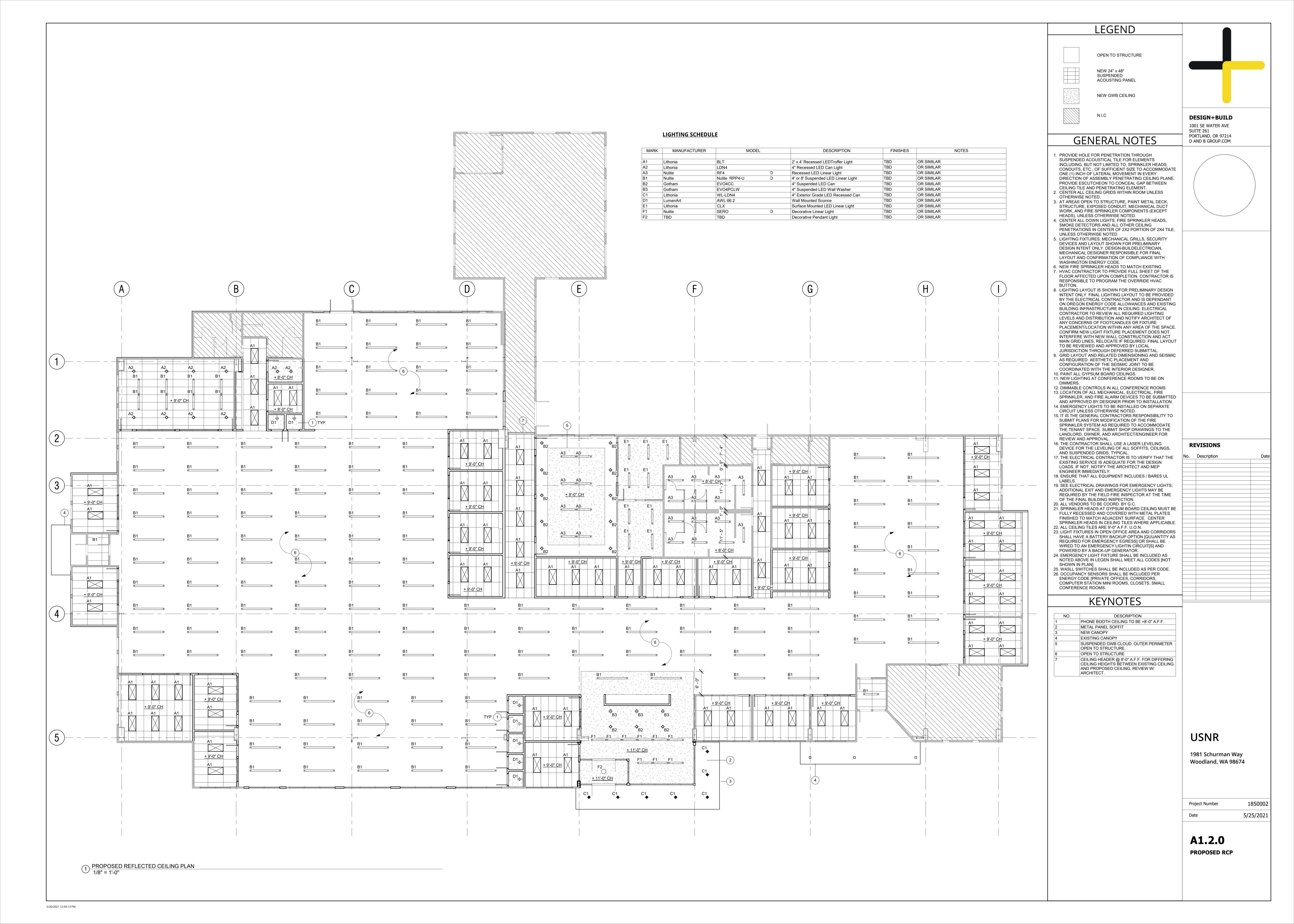


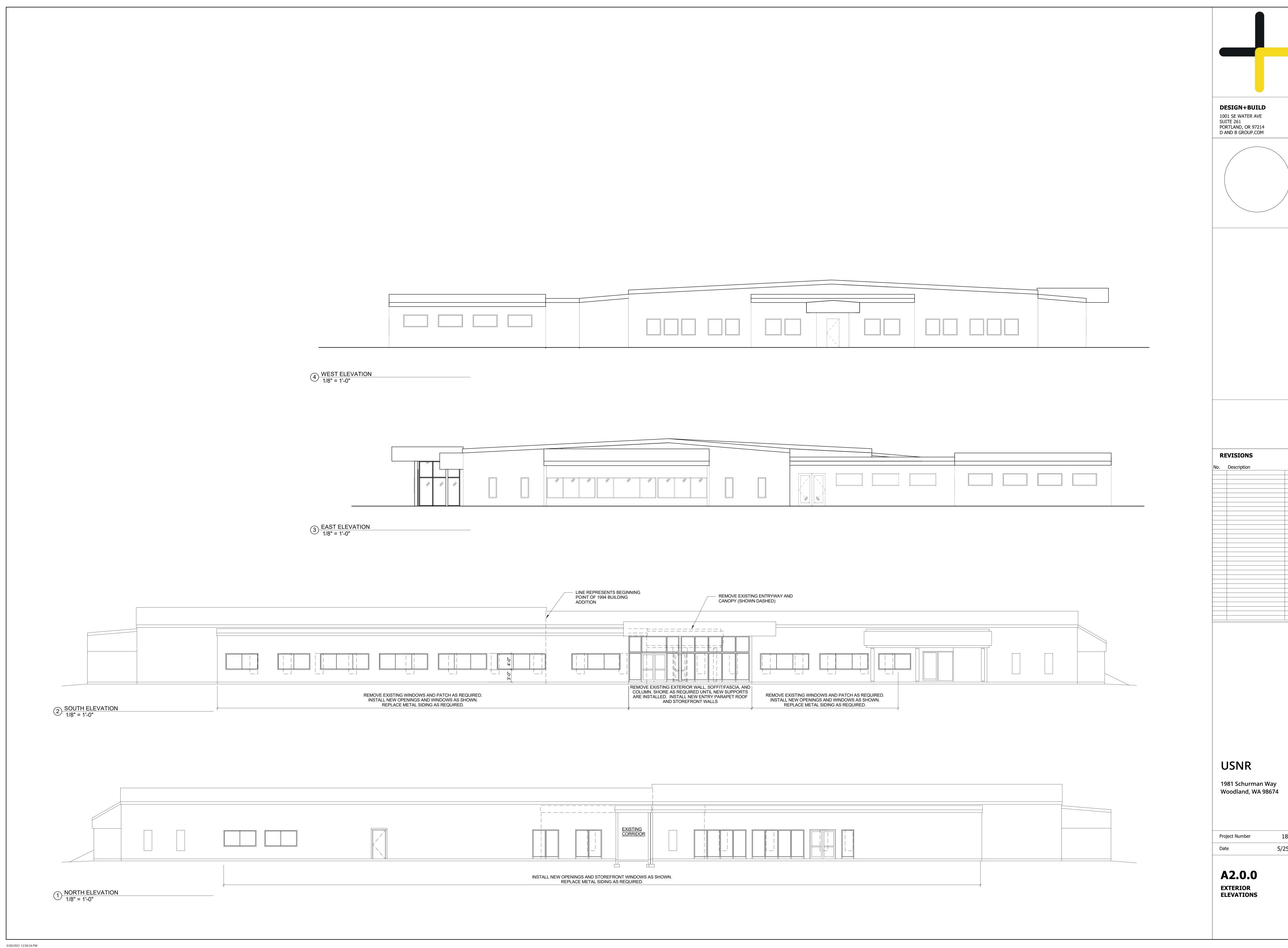


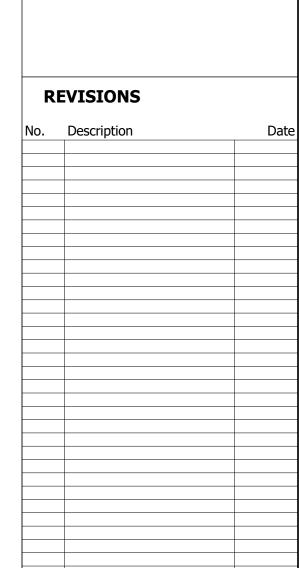


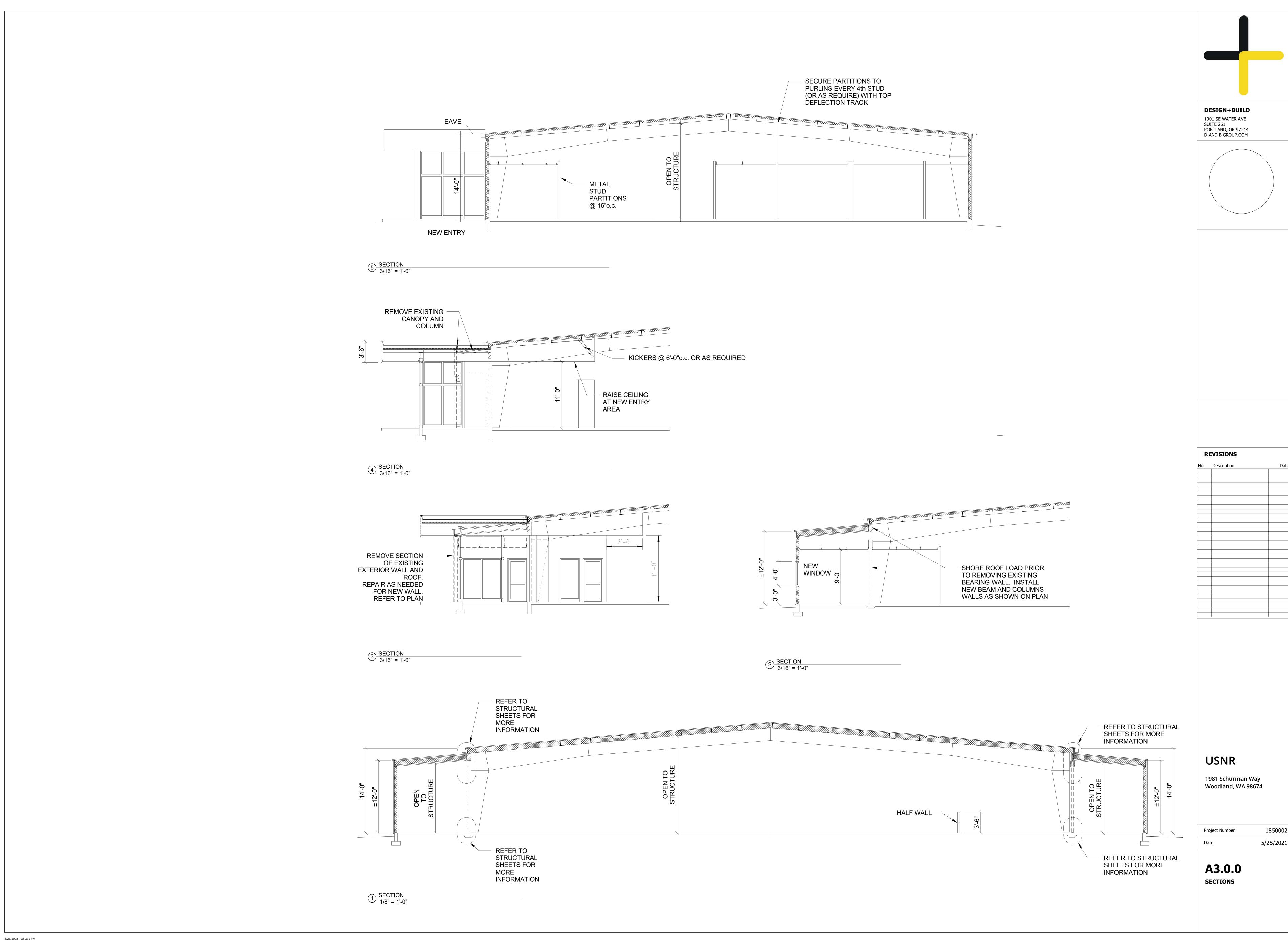


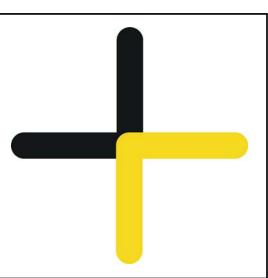


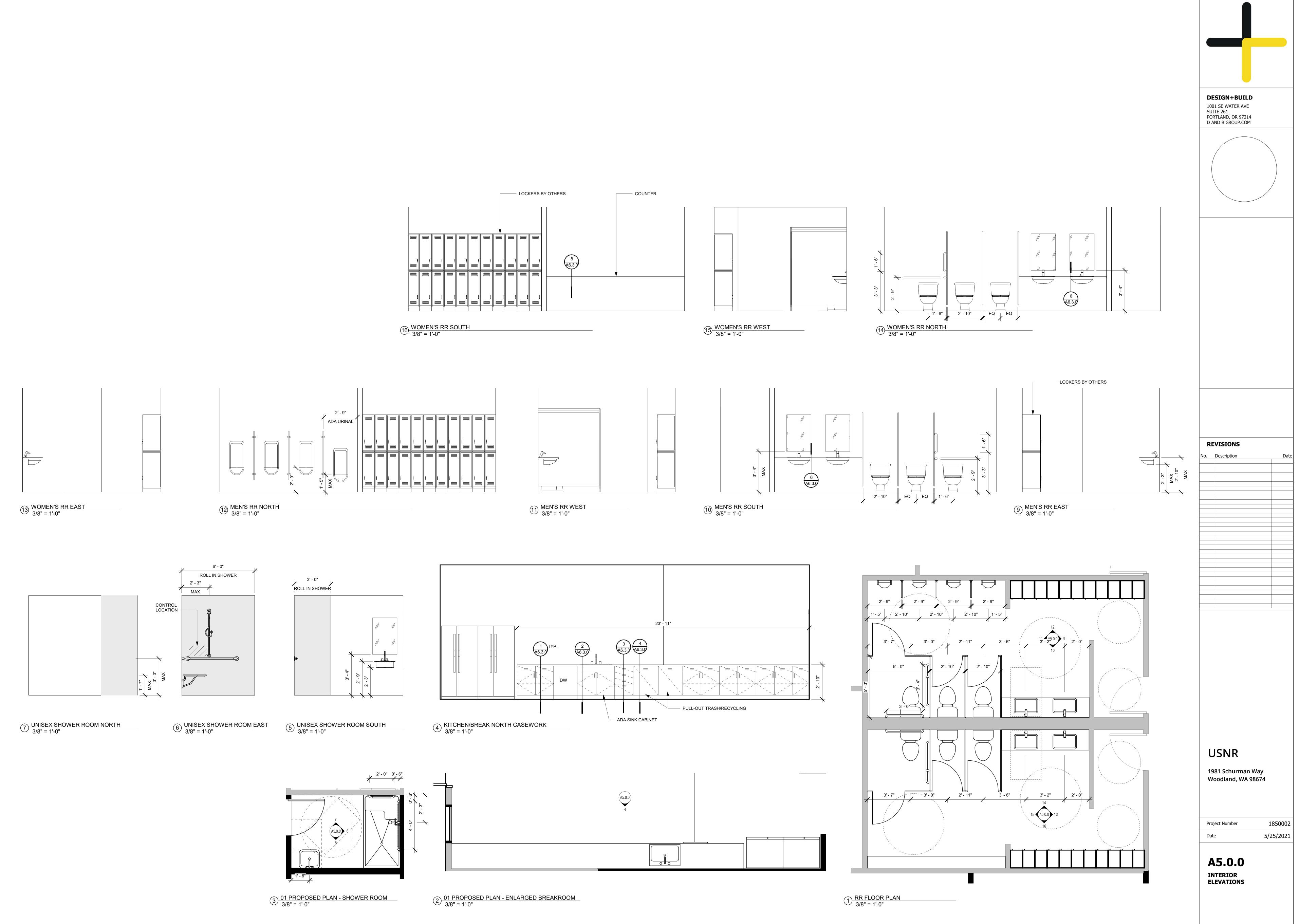


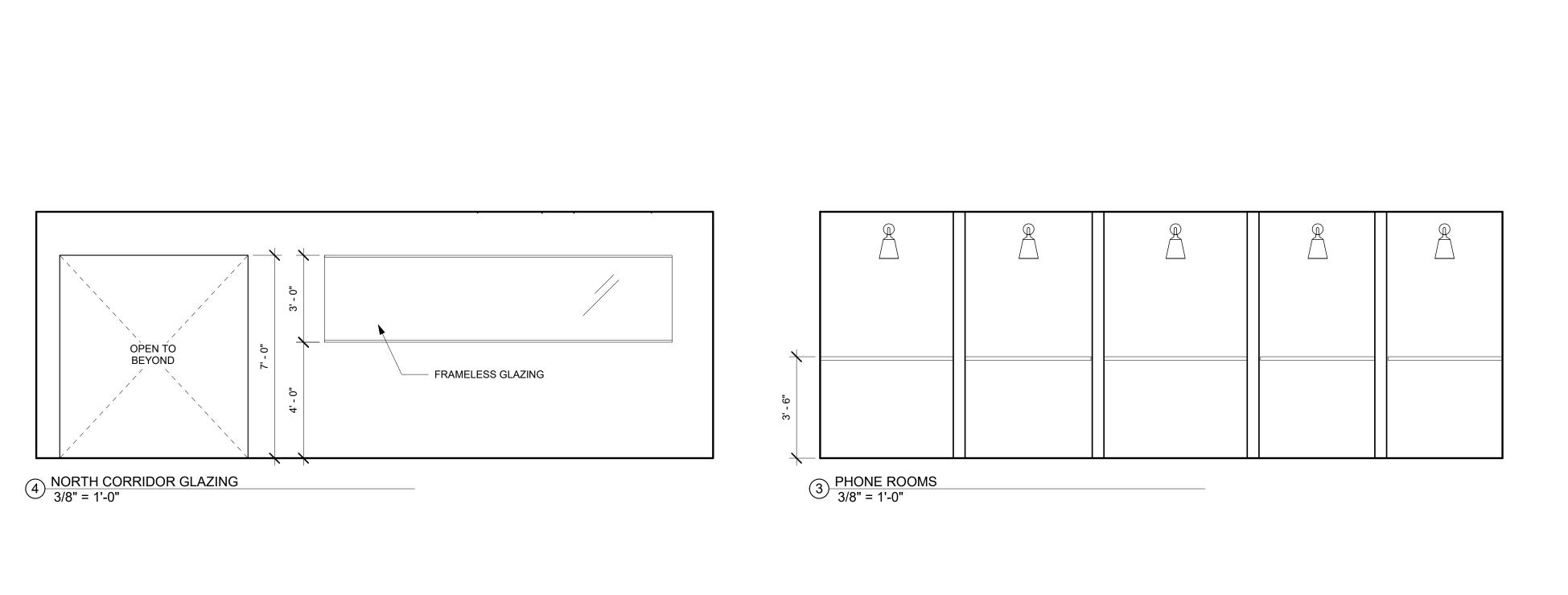


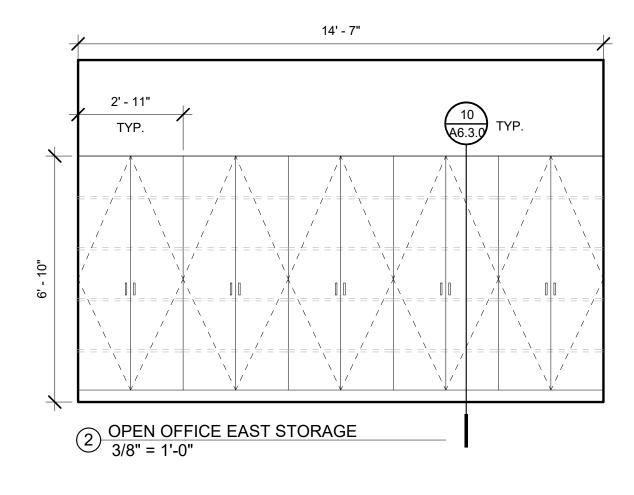




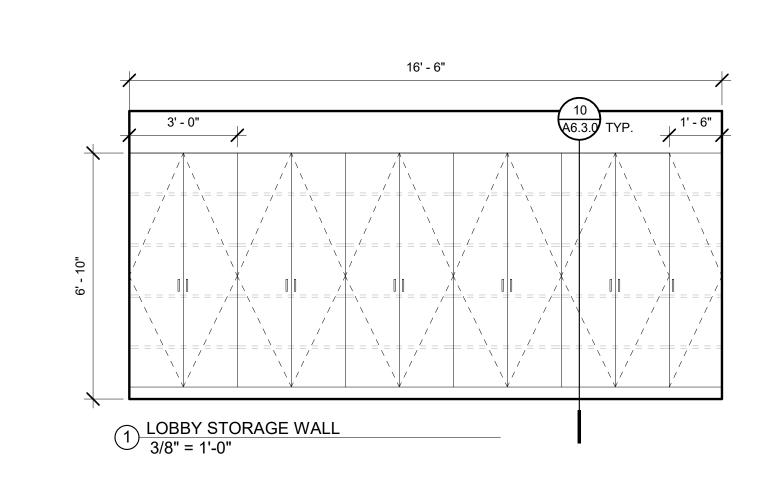


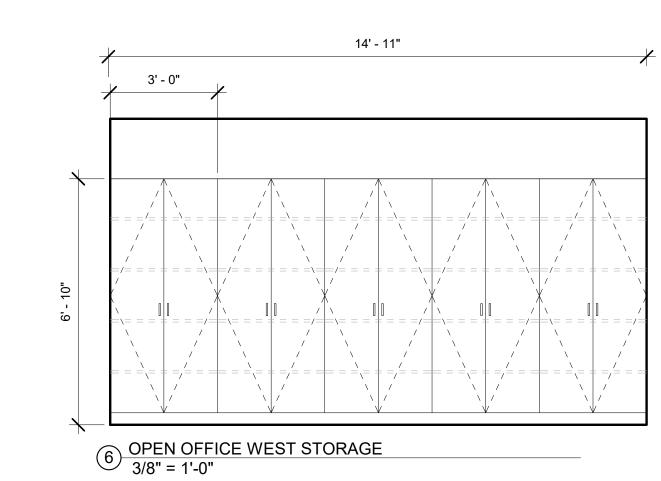


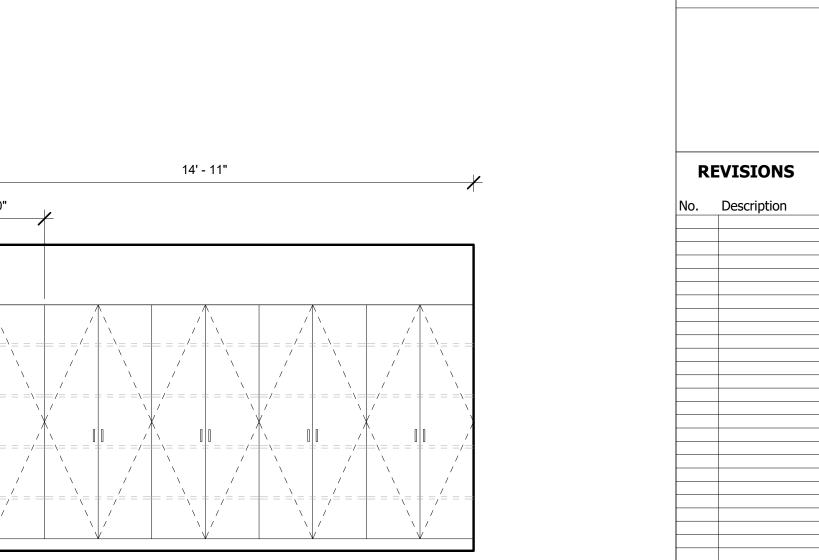




7 BREAKROOM STORAGE
3/8" = 1'-0"







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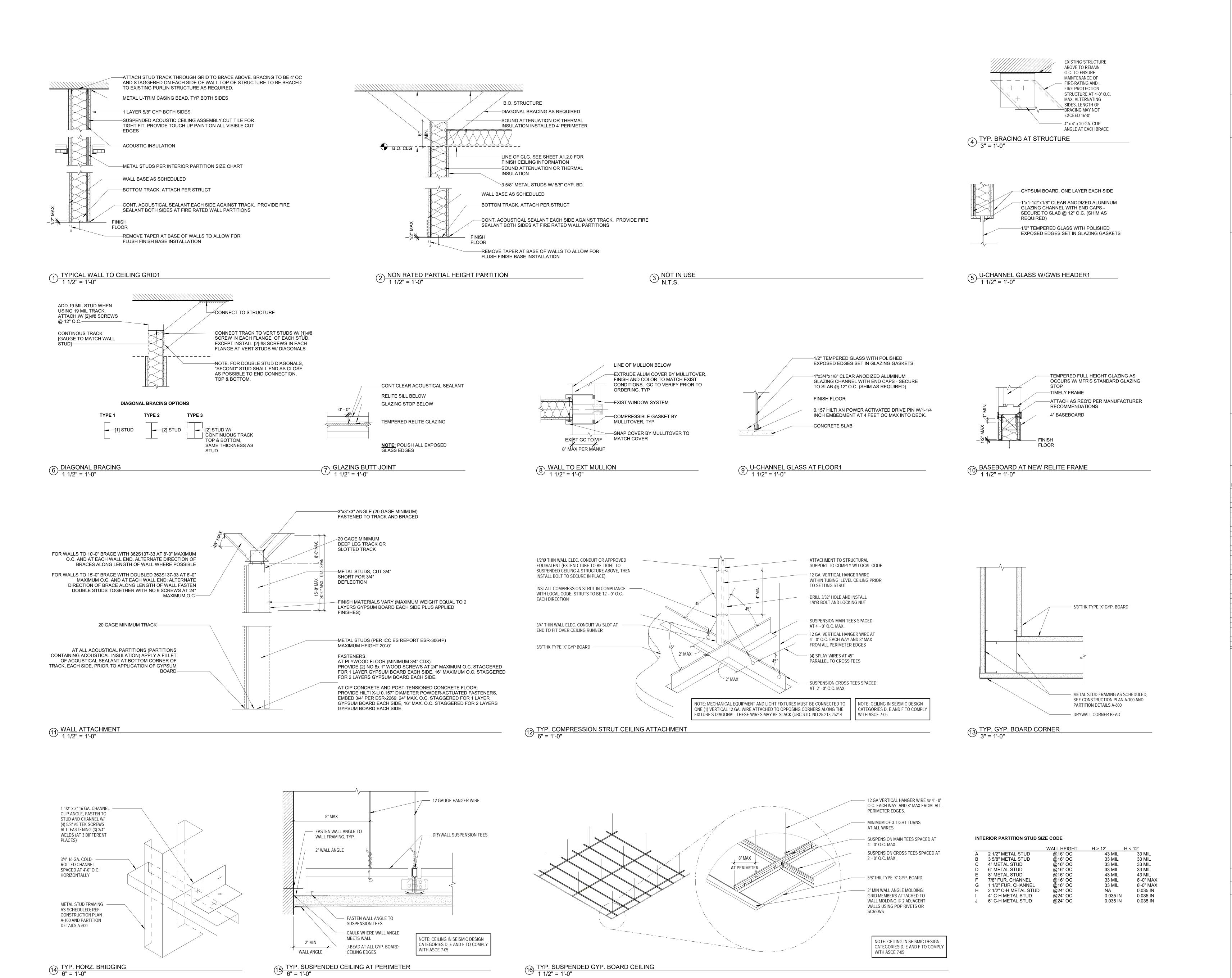
USNR

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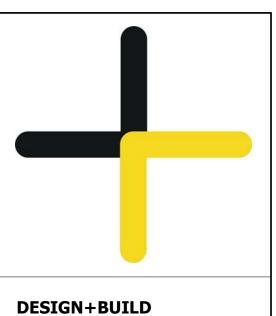
1850002 Project Number 5/25/2021

A5.1.0 INTERIOR ELEVATIONS

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SUITE 261

Description

REVISIONS

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Project Number 1850002

Date 5/25/2021

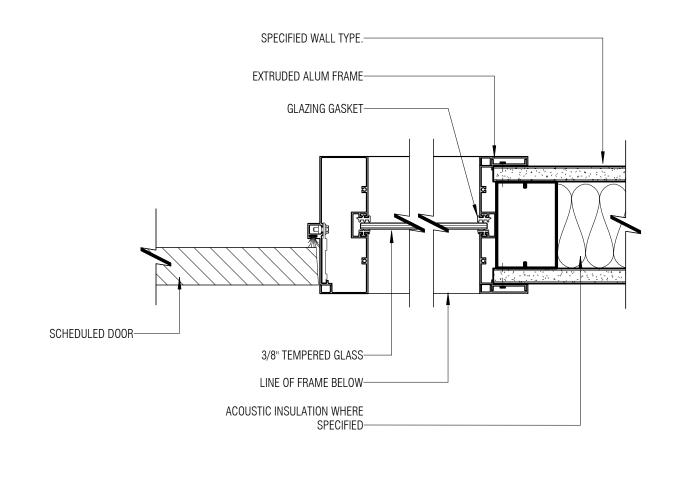
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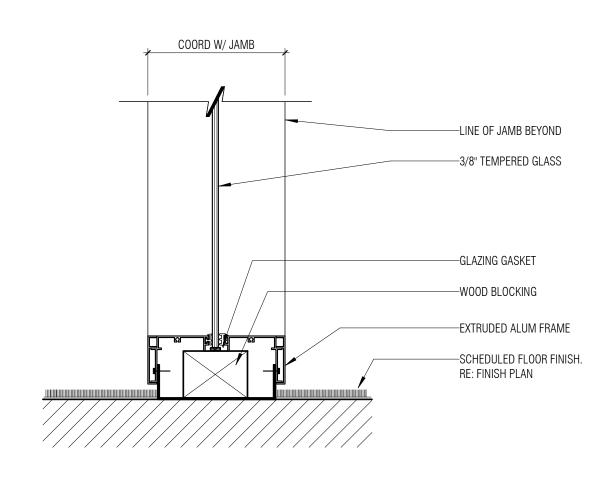
WALLS + CEILING DETAILS

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3 ALUMINUM DOOR JAMB WITH SIDELIGHT
3" = 1'-0"

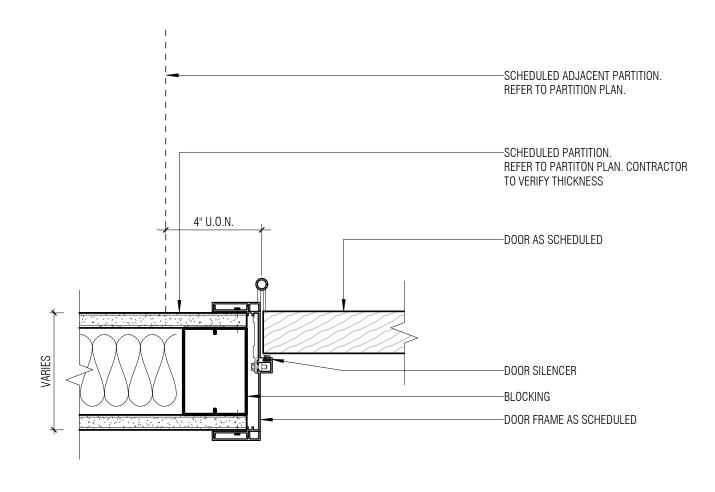
6 ALUMINUM GLAZING SILL 3" = 1'-0"

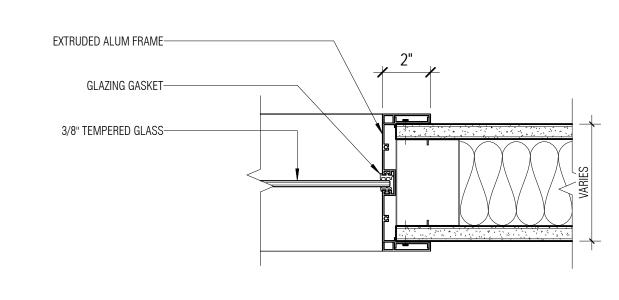




2 ALUMINUM DOOR JAMB
3" = 1'-0"

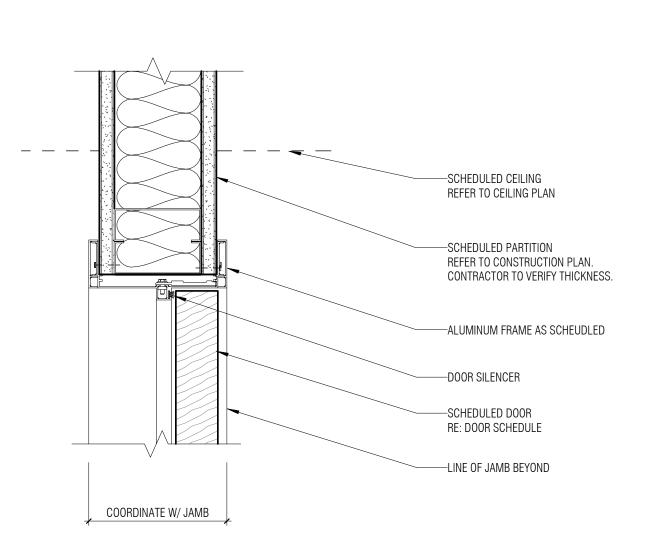
5 ALUMINUM FRAME GLAZING JAMB
3" = 1'-0"

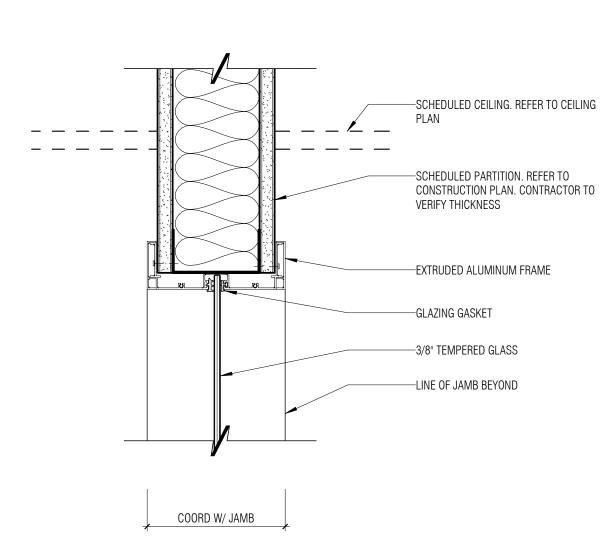




1 ALUMINUM DOOR HEAD 3" = 1'-0"

4 ALUMINUM GLAZING HEAD
3" = 1'-0"





DOOR + GLAZING DETAILS

A6.1.0

1850002

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USNR

Project Number

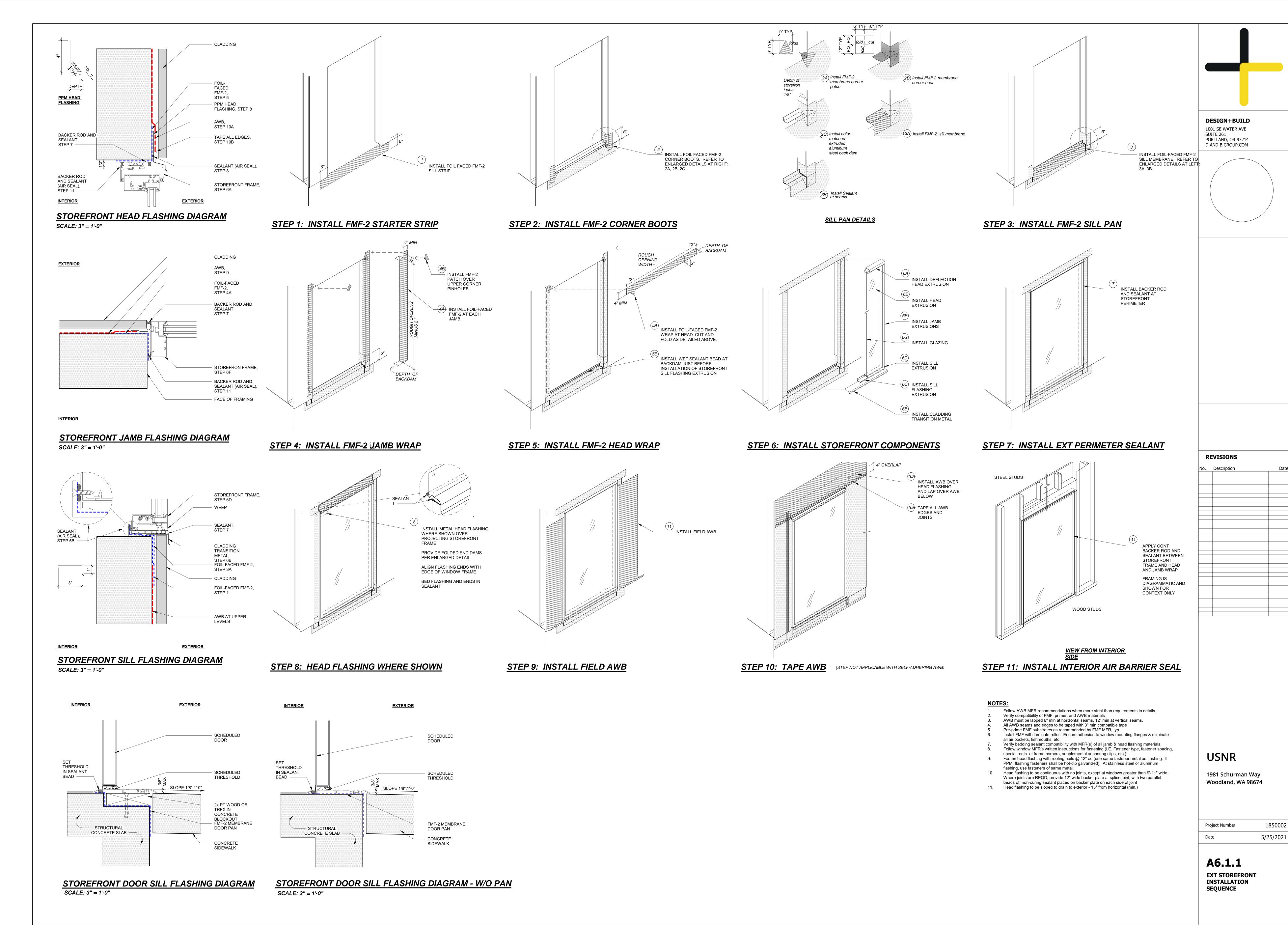
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REVISIONS

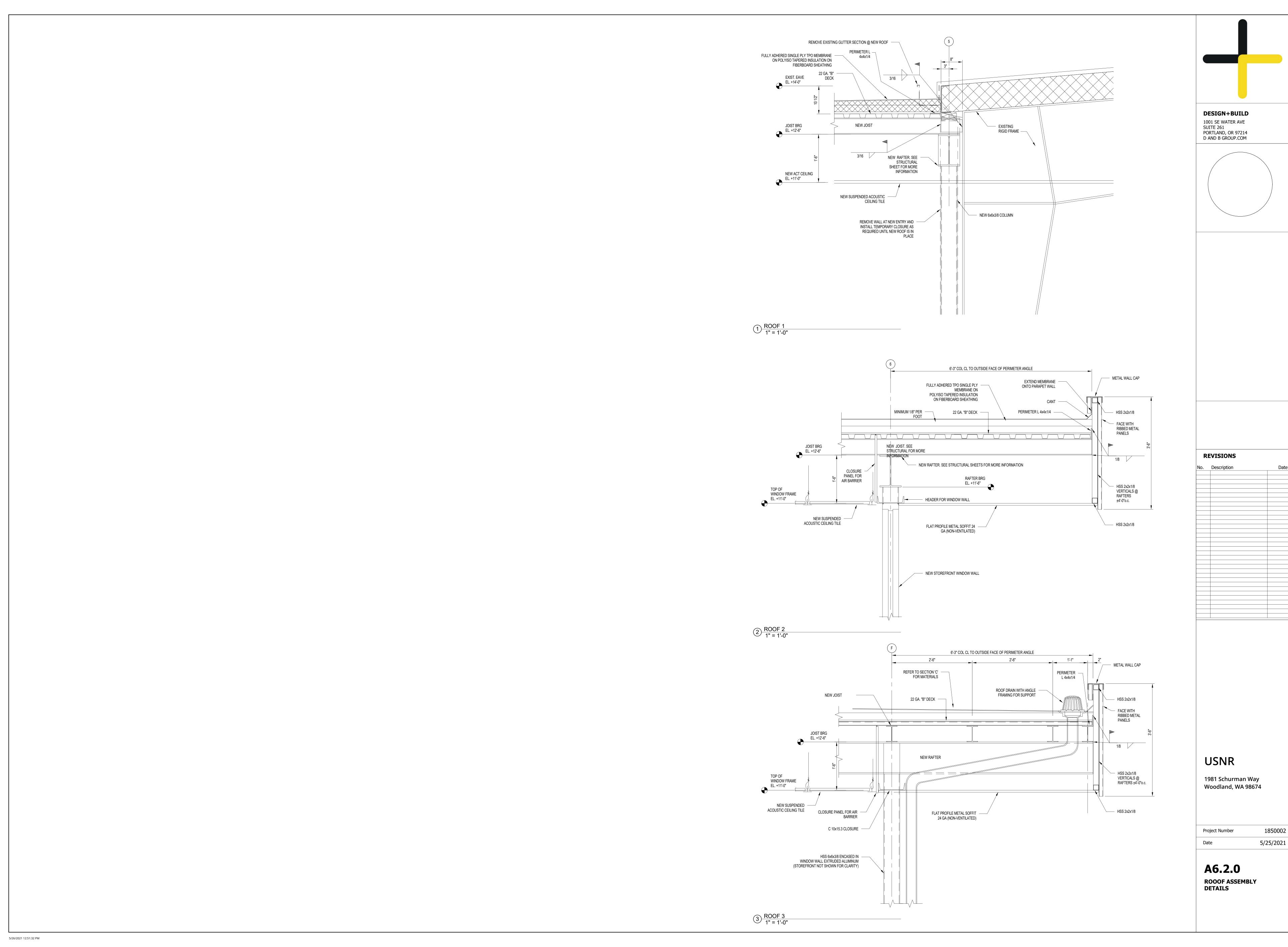
DESIGN+BUILD

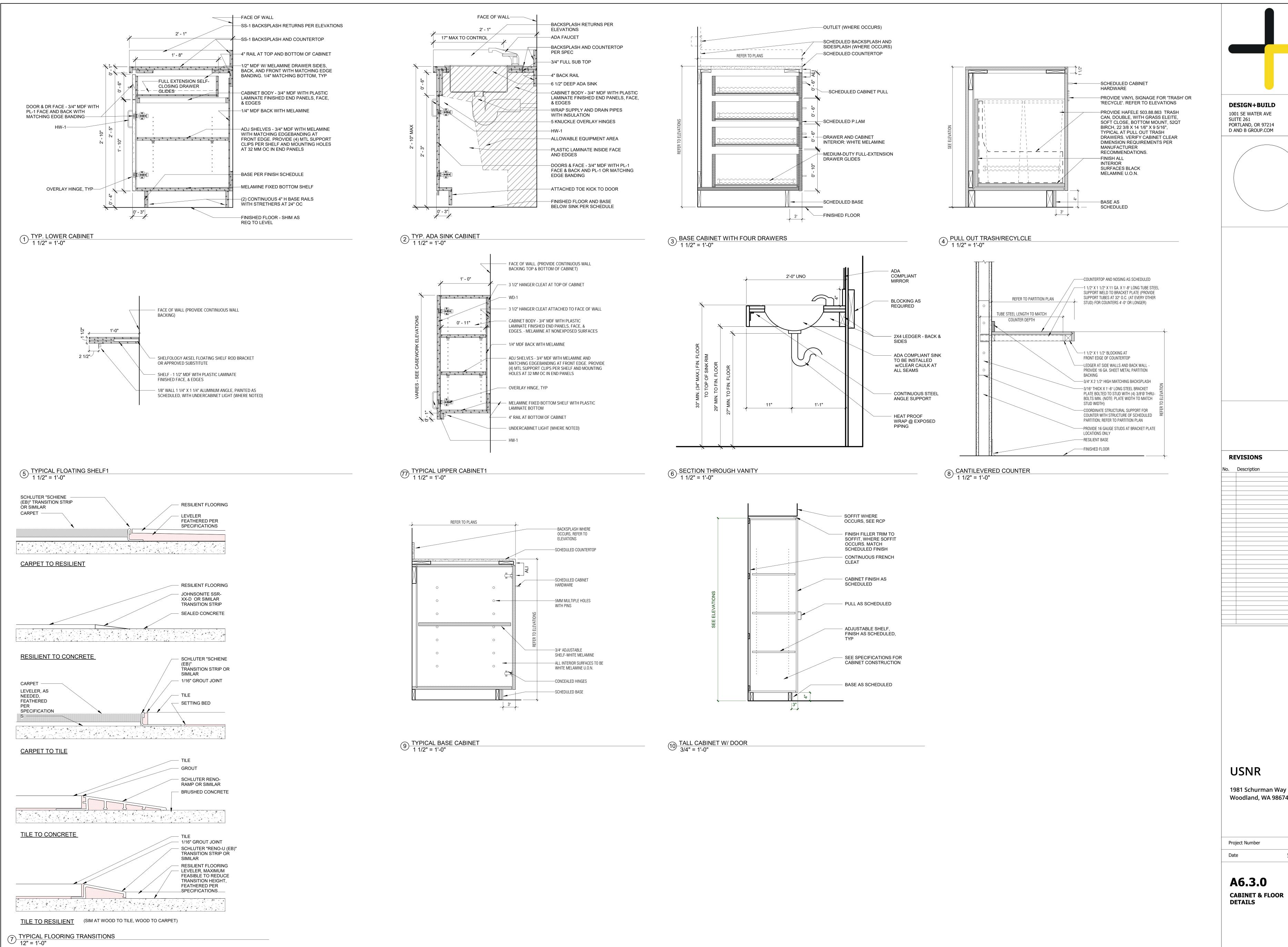
1001 SE WATER AVE

SUITE 261
PORTLAND, OR 97214
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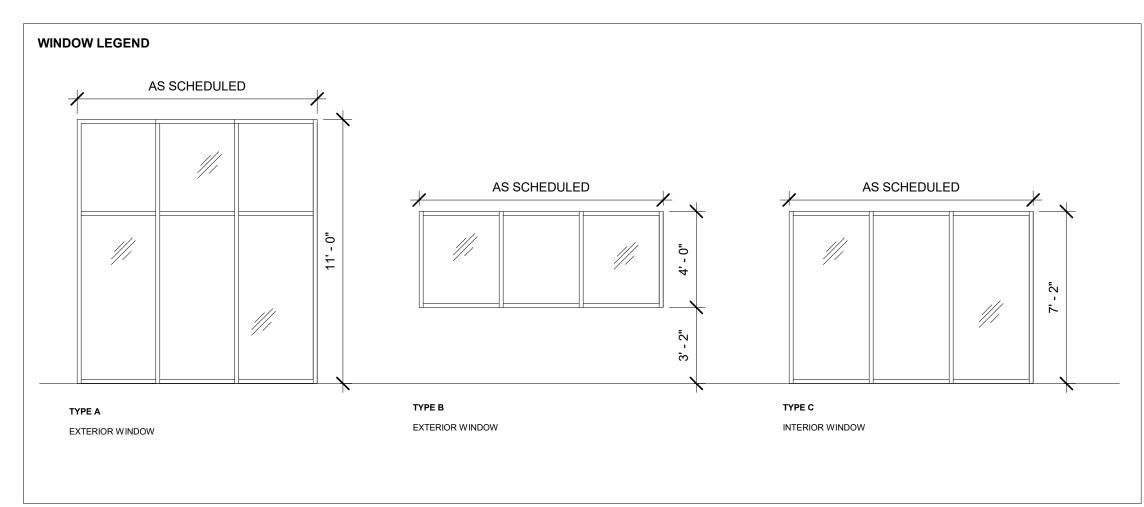




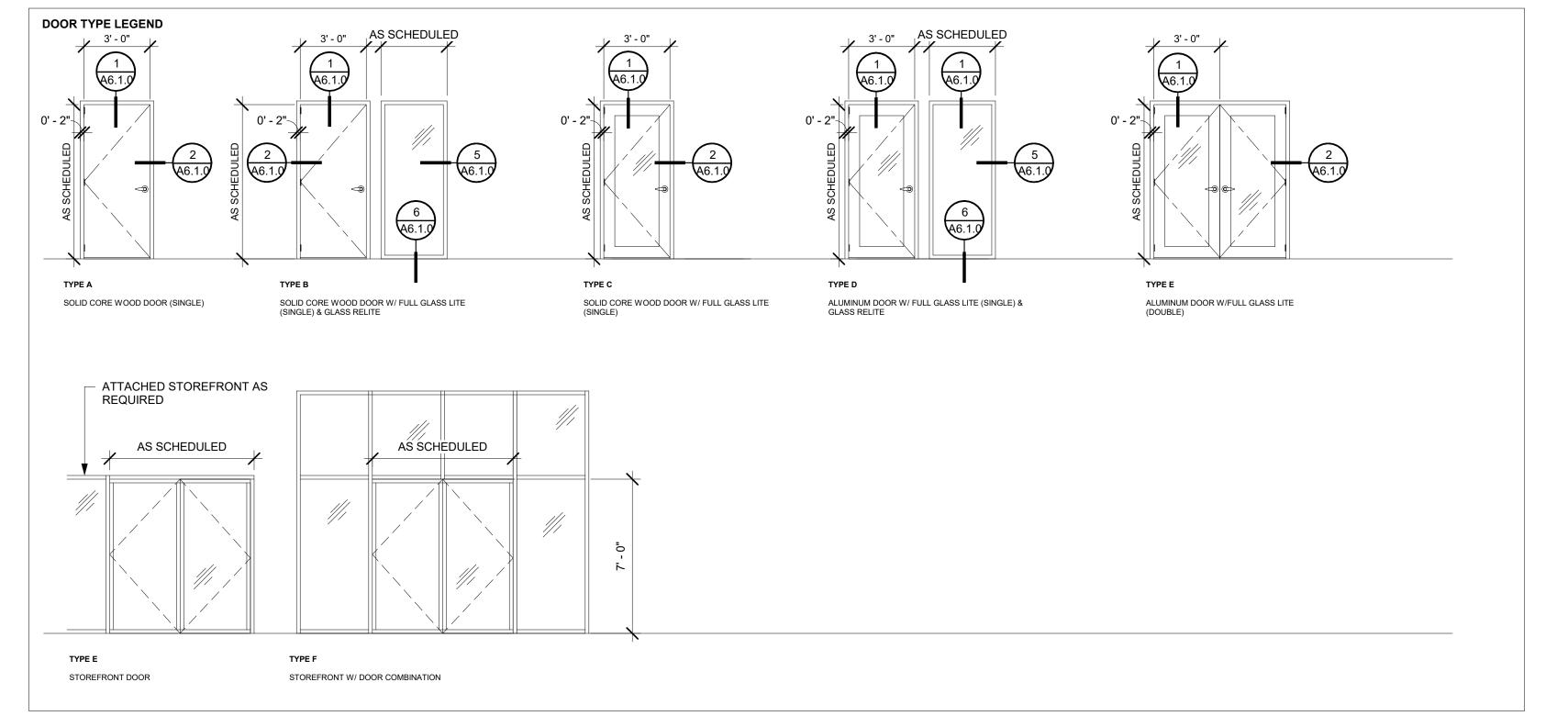
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Woodland, WA 98674

WI	WINDOW SCHEDULE								
	STOREFRONT								
Mark	Type Mark	Width	Height	Description	Finish	Comments			
1	В	8'-0"	4' - 0"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
2	В	12'-0"	4' - 0"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
3	Α	16-'7"	11' - 0"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
4	Α	10'-0"	11' - 0"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
5	С	6'-6"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
6	С	12'-6"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
7	С	3'-0"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
8	С	6'-6"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
9	С	10'-0"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
10	С	9'-0"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
11	С	12'-0"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
12	С	5'-0"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			
13	С	4'-0"	7' - 2"	1 3/4" X 4 1/2" MULLIONS W/ 1/4" CENTER GLAZING	Anodized Aluminum	OR SIMILAR			

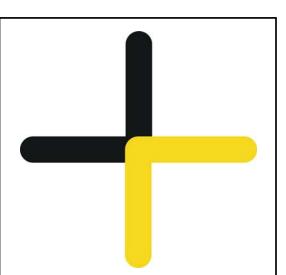


			JLE DOOR								FRAME OPENING FIRE			
NO.	ROOM NAME	WIDTH	HEIGHT	THICKNESS		TYPE	CORE	MATERIAL	FINISH	TYPE	MATERIAL	FINISH	RATING	NOTES
Оа	ENTRY	6' - 0 1/4"	6' - 11"		1	F		METAL	ALUMINUM	METAL	ALUMINUM			
	EXIT	3' - 0"	7' - 0"	1 3/4"	(none)	-		-	-	-	ALOMINOM		_	EXISTING TO REMAIN
	EXIT	3' - 0"	7' - 0"	1 3/4"	(none)			-	- -				_	EXISTING TO REMAIN
	HALLWAY	6' - 6"	8' - 0"	1 3/4"	(none)		<u>-</u>	-	-	- -		_	_	EXISTING TO REMAIN
	KITCHEN / BREAK	6' - 0"	7' - 0"	1 3/4"	,	F	-	METAL	ALUMINUM	METAL	ALUMINUM			EXISTING TO REMAIN
	EXIT	3' - 0"	7' - 0"	1 3/4"	(none)	_		-	-	-	ALOMINOM	_	_	EXISTING TO REMAIN
	LOBBY	6' - 0 1/4"	6' - 11"	1 0/4	` '	F		METAL	ALUMINUM	METAL	ALUMINUM			EXISTING TO REMAIN
	OFFICE	3' - 0"	7' - 0"	1 3/4"	•	В	SOLID	WOOD	TBD	METAL	ALUMINUM			+
	OFFICE	3' - 0"	7' - 0"	1 3/4"	_	В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"	_	В	SOLID	WOOD	TBD	METAL	ALUMINUM			+
	CONFERENCE ROOM	3' - 0"	7' - 0"	1 3/4"		В	OOLID	METAL	ALUMINUM	METAL	ALUMINUM			
	CONFERENCE ROOM	3' - 0"	7' - 0"	1 3/4"		В		METAL	ALUMINUM	METAL	ALUMINUM			+
	EXG. CONFERENCE ROOM	3' - 0"	7' - 0"	1 3/4"		С		METAL	ALUMINUM	METAL	ALUMINUM			
	EXG. CONFERENCE ROOM	3' - 0"	7' - 0"	1 3/4"	(none)	<u> </u>		IVILIAL	ALOWINOW	IVILIAL	ALOMINOM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"	` '	В	SOLID	WOOD	TBD	METAL	ALUMINUM			+
	OFFICE	3' - 0"	7 - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			+
	OFFICE	3' - 0"	7 - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			+
	OFFICE	3' - 0"	7 - 0"	1 3/4"	-	В	SOLID	WOOD	TBD	METAL	ALUMINUM			_
	OFFICE	3' - 0"	7' - 0"	1 3/4"	(none)	ט	JOLID	VVOOD	טטו	IVIL I AL	ALUIVIIINUIVI			
	OFFICE	3' - 0"	7' - 0"	1 3/4"	· '									
	OFFICE	3' - 0"	7' - 0"	1 3/4"	(none)	В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	EXG. MECHANICAL ROOM	3' - 0"	7 - 0"			В	SOLID	WOOD	עסו	METAL	ALUMINUM			EXISTING TO REMAIN
	HALLWAY	6' - 6"	7 - 0"	1 3/4"	(none)	-	-	-	-	-	-	-	-	
			7 - 0"	4.2/4"	(none)	-	-	-	-	-	-	-	-	DRYWALL WRAPPED OPENING
	EXG. MECHANICAL ROOM	3' - 0"	_	1 3/4"	(none)	-	-	-	-	-	-	-	-	EXISTING TO REMAIN
	EXG. RR	3' - 0"	7' - 0"	1 3/4"	(none)	-	-	-	-	-	-	-	-	EXISTING TO REMAIN
	EXG. RR	3' - 0"	7' - 0"	1 3/4"	(none)	<u>-</u>	-	-	-	-		-	-	EXISTING TO REMAIN
	OFFICE	3' - 0"	7' - 0"	1 3/4"	-	В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"	_	В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	STORAGE	3' - 0"	7' - 0"	1 3/4"	-	A	SOLID	WOOD	TBD	METAL	ALUMINUM			
	EXG. SERVER	3' - 0"	7' - 0"	1 3/4"	_	A	SOLID	WOOD	TBD	METAL	ALUMINUM			
	STORAGE	3' - 0"	7' - 0"	1 3/4"		A	SOLID	WOOD	TBD	METAL	ALUMINUM			
	CONFERENCE ROOM	3' - 0"	7' - 0"	1 3/4"		В		METAL	ALUMINUM	METAL	ALUMINUM			
	CONFERENCE ROOM	3' - 0"	7' - 0"	1 3/4"	· ,	В	2011							
	OFFICE	3' - 0"	7' - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	PHONE	2' - 10"	7' - 0"	1 3/4"		С		METAL	ALUMINUM	METAL	ALUMINUM			
	PHONE	2' - 10"	7' - 0"	1 3/4"		С		METAL	ALUMINUM	METAL	ALUMINUM			
	STORAGE	3' - 0"	7' - 0"	1 3/4"	_	Α	SOLID	WOOD	TBD	METAL	ALUMINUM			
	SHOWER ROOM	3' - 0"	7' - 0"	1 3/4"	_	A	SOLID	WOOD	TBD	METAL	ALUMINUM			
	EXG. RR	3' - 0"	7' - 0"	1 3/4"	(none)	-	-	-	-	-	-	-	-	EXISTING TO REMAIN
	EXG. RR	3' - 0"	7' - 0"	1 3/4"	(none)	-	-	-	-	-	-	-	-	EXISTING TO REMAIN
	CONFERENCE	3' - 0"	7' - 0"	1 3/4"		В		METAL	ALUMINUM	METAL	ALUMINUM			
	CONFERENCE	3' - 0"	7' - 0"	1 3/4"	4	В		METAL	ALUMINUM	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	CONFERENCE	3' - 0"	7' - 0"	1 3/4"		В	SOLID	WOOD	TBD	METAL	ALUMINUM			
a	OFFICE	3' - 0"	7' - 0"	1 3/4"	3	В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	OFFICE	3' - 0"	7' - 0"	1 3/4"	_	В	SOLID	WOOD	TBD	METAL	ALUMINUM			
	PHONE	2' - 10"	7' - 0"	1 3/4"	4	С		METAL	ALUMINUM	METAL	ALUMINUM			
Вс	EXG. SERVER	3' - 0"	7' - 0"	1 3/4"	(none)									
la	PHONE	2' - 10"	7' - 0"	1 3/4"	4	С		METAL	ALUMINUM	METAL	ALUMINUM			
5а	PHONE	2' - 10"	7' - 0"	1 3/4"	4	С		METAL	ALUMINUM	METAL	ALUMINUM			
3a	PHONE	2' - 10"	7' - 0"	1 3/4"	4	С		METAL	ALUMINUM	METAL	ALUMINUM			
7a	PHONE	2' - 10"	7' - 0"	1 3/4"	4	С		METAL	ALUMINUM	METAL	ALUMINUM			
	OPEN OFFICE	3' - 0"	7' - 0"	1 3/4"	(none)	-	-	-	-	-		-	-	EXISTING TO REMAIN
Оа	EXIT	3' - 0"	7' - 0"	1 3/4"	(none)	-	-	-	-	-		-	-	EXISTING TO REMAIN
	OFFICE	3' - 0"	7' - 0"	1 3/4"	(none)	-	-	-	-	-		_	-	EXISTING TO REMAIN
	HALLWAY	3' - 0"	7' - 0"	1 3/4"	(none)	_	1_							EXISTING TO REMAIN



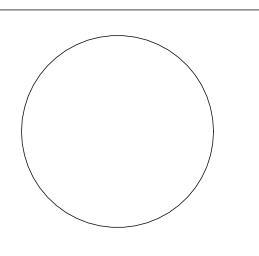
DOOR HARDWARE LEGEND

- HW1 ENTRY LOCK (THUMBLATCH) W/EGRESS LATCH, SCHLAGE (OR APPROVED ALTERNATE), FINISH TBD
- HW2 RESTROOM PRIVACY LATCH W/OCCUPANCY INDICATOR, SCHLAGE (OR APPROVED ALTERNATE), FINISH TBD
- HW3 OFFICE LATCH WITH EGRESS LEVER, SCHLAGE (OR APPROVED ALTERNATE), FINISH TBD
- HW4 PASSAGE LATCH W/EGRESS LEVER BOTH SIDES, SCHLAGE (OR APPROVED ALTERNATE), FINISH TBD
- HW5 PUSH/PULL PLATES W/CLOSER (NO LATCH)
- HW6 ELECTRONIC LOCK W/KEYCARD READER, LOCK TO BE FAILSAFE TO RELEASE UPON LOSS OF POWER OR ACTIVATION OF FIRE ALARM, CLOSER



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No. Description Date

USNR

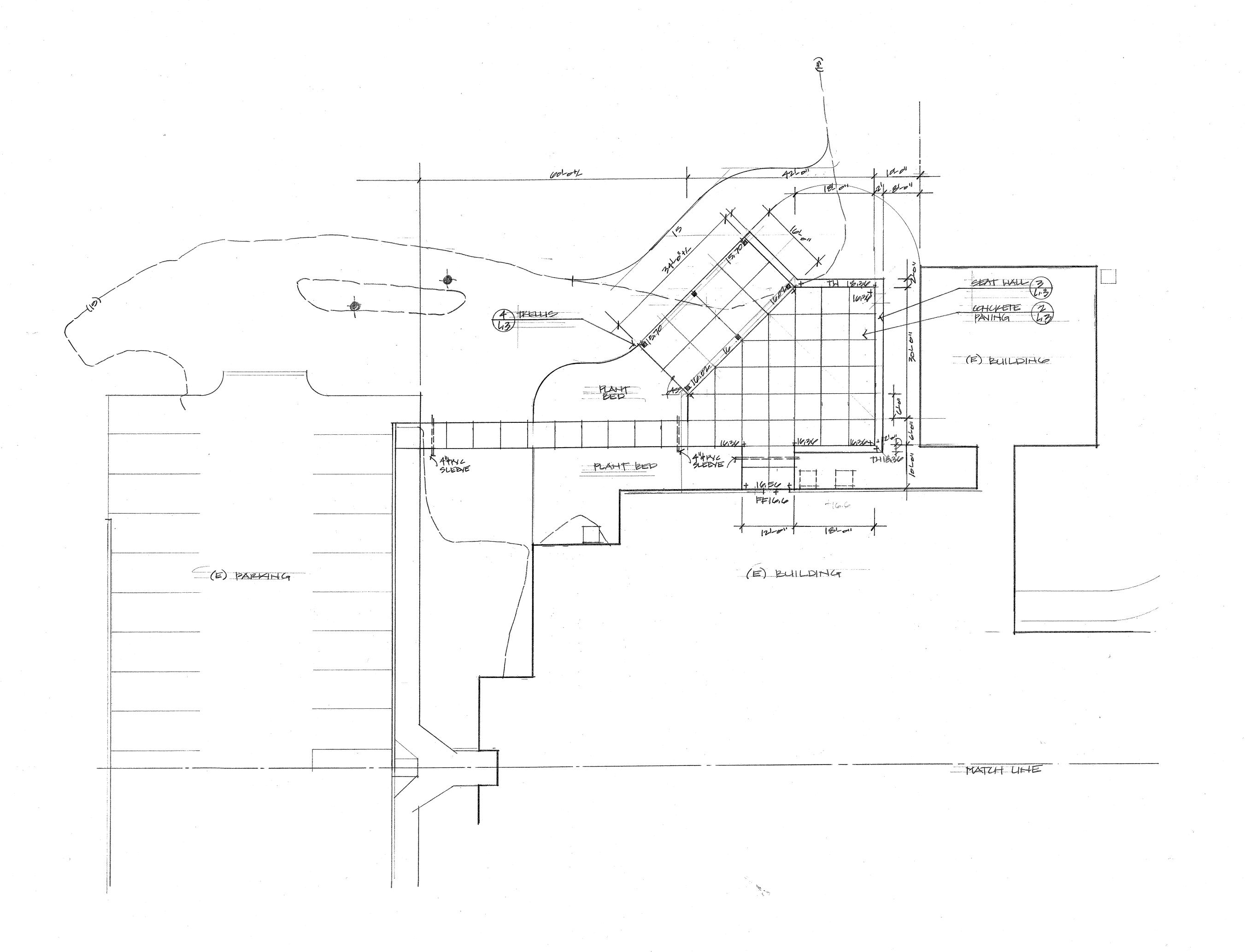
1981 Schurman Way Woodland, WA 98674

Project Number 1850002

Date 5/25/2021

A7.0.0

DOOR & WINDOW SCHEDULE







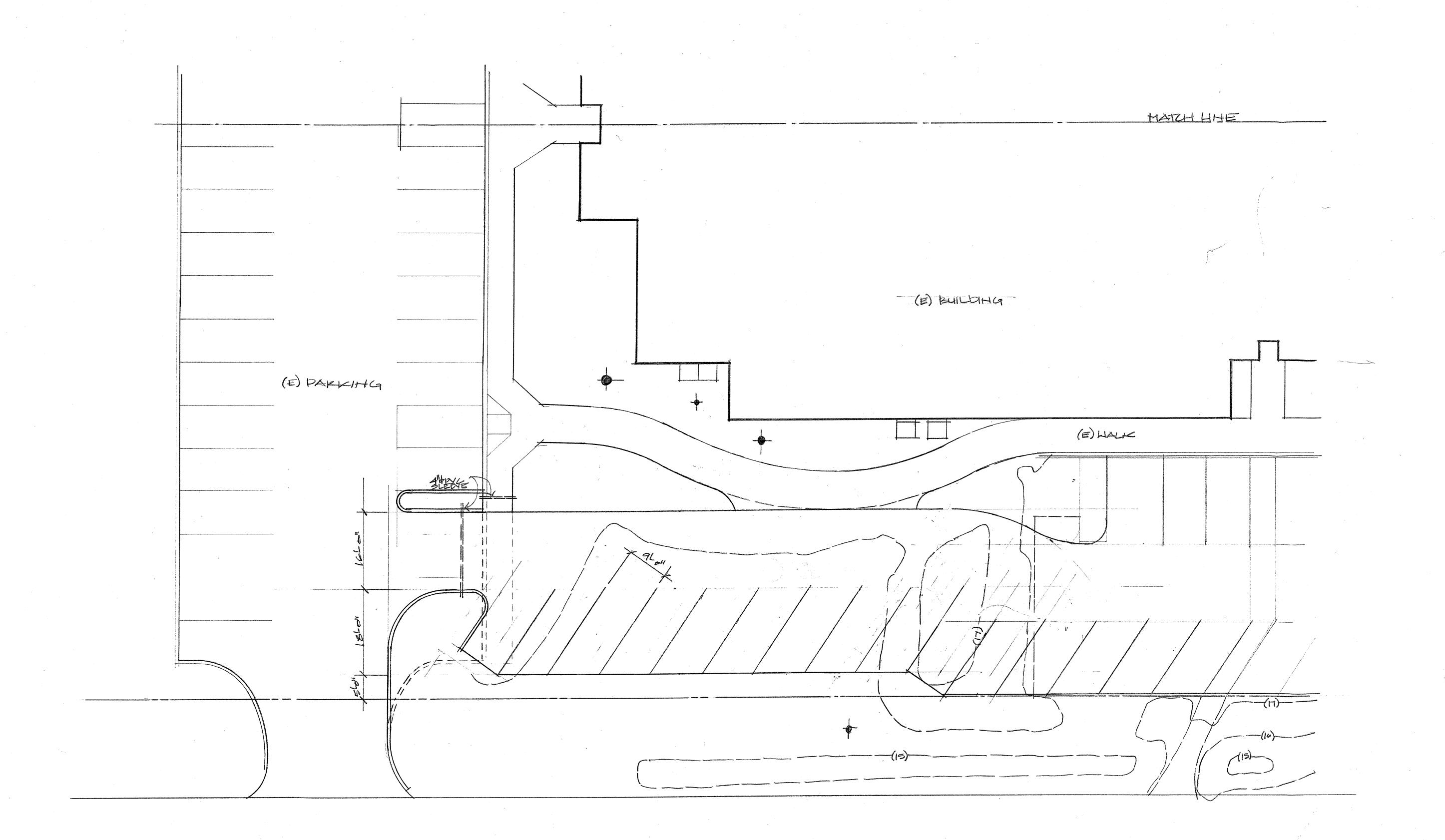
BRIAN BAINNSON

CERTIFICATE NO. 717

USNR Offices
1981 Schurman Way
Woodland, Washington 98674

TERRACE

REVISIONS:







BRIAN BAINNSON
CERTIFICATE NO. 717

USNR Offices
1981 Schurman Way
Woodland, Washington 98674

PARKING AKEA PLAN

REVISIONS: 5,24,2021

DATE: 5,18,2



Tel: (503) 256-8955



EXIOSCAPE ARCHITE

BRIAN BAINNSON CERTIFICATE NO. 717

USNR Offices
1981 Schurman Way
Woodland, Washington 98674

DETAILS

REVISIONS:

DATE:

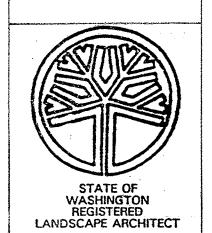
5/18/2021

SCALE:

AS SHOW

SHEET:





Tel: (503) 256-8955

BRIAN BAINNSON

BRIAN BAINNSON CERTIFICATE NO. 717

USNR Offices
1981 Schurman Way
Woodland, Washington 98674

PLANTING PLANTING

REVISIONS:

5,24,2021

DATE:

SCALE:

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STRUCTURAL NOTES

GENER A

1. These notes set minimum standards for construction. The drawings govern over these notes to the extent shown. Coordinate these drawings with architectural specifications and notify Lewis & Van Vleet Inc. Engineers (LVI) of any discrepancies prior to beginning work.

These drawings have been prepared solely for use in construction of the USNR Remodel project located in Woodland, Washington. Possession of these drawings does not grant license to construct or fabricate the whole or parts of this project in other locations.
 The contractor shall verify all dimensions and conditions on drawings and in field. Coordinate locations of openings through floors, roofs, and walls with architectural, mechanical, plumbing and electrical drawings. Notify engineer of any discrepancies.
 The contractor shall be responsible for providing all temporary support prior to completion of the vertical and lateral load systems. LVI has not been retained to provide any services pertaining to job site safety precautions, or to review means, methods techniques, sequences, or procedures for performing the work. Unless we are specifically retained and compensated to do otherwise, our work is limited to the design of work described on our drawings.
 Where reference is made to ACI, AISC, ASTM, or other standards or codes, the latest

edition shall apply.
6. Inspection and or job supervision is not provided by LVI.
7. All work shall be in strict compliance with the latest edition of the International Building Code (IBC) and all other state and local codes which apply
8. Any mechanical equipment, piping, ductwork, etc. which applies a load of 150 pounds or more shall be hung from a system approved by LVI.

DESIGN CRITERIA

- a. Design Snow Load = 25 psf rain on snow
 b. Drift lengths, loads and locations per roof framing plan.
- 2. Wind:
 a. Basic Wind Speed: ASCE 7—10 110 mph (ultimate) Cowlitz County, Washington
 b. Occupancy Category II, Risk Category II
- c. Exposure: B
 d. Internal pressure coefficient GCPI = (+/-) 0.18
- 3. Seismic: a. Risk Category II
- b. Seismic Importance Factor: IE = 1.0
 c. Mapped Spectral Response parameters SS = 0.911, S1 = 0.412
- d. Site class D (Default) e. Design Spectral Response parameters: SDS = 0.690, SD1 = 0.436 f. Seismic Design Category D
- g. Design Seismic force—resisting system being modified: Wood Shear Panel Walls
 h. Seismic Response Coefficient CS = 0.106
- i. Response Modification Coefficient R = 6 1/2
 j. Analysis Procedure: Equivalent Lateral Force

FOUNDATIONS

Design soil bearing pressure equals 1500 psf live plus dead load
 All footings to bear on firm, undisturbed native soils or structural fill a minimum of 12" below finish exterior grade. Notify engineer before proceeding if any unusual conditions are encountered in footing excavations.

3. Do not excavate closer than 2:1 slope adjacent to footing excavations.
4. Clean all footing excavations of loose material by hand. Remove all wet, soft soil from footing excavations prior to placing concrete.
5. Excavations may be made under footings for pipes. Backfill to be "structural fill" as defined above.

CONCRETE

1. Average concrete strength to be as indicated below and determined by job cast lab cured cylinder at 28 days plus increase depending on plant's standard deviation as specified in ACI 318. Provide mix designs to engineer for review prior to placing any concrete. CLEARLY LABEL ALL MIX DESIGNS AS TO PROPOSED AREA OF USE. Supplier to label all mix designs with an identification number. Mix number should be referenced in all subsequent concrete test reports.

2. Minimum mix requirements:

Location	Compressive strength (psi)	Minimum cement content	Admixtures
Footings	3000	5	none
Slabs on grade (interior)	3500	5 1/2	WRA (a)
Slabs on grade (exterior)	3000	5 1/2	WRA,AE (b.)
Miscellaneous	3000	5	none

- a. WRA= Water Reducing Admixture
- b. AE= Air Entrainmentc. Provide an accelerator in all concrete placed below 40 degrees.
- 3. Use Type I cement, per ASTM C-150 unless otherwise approved. Water cement ratio to be 0.46 maximum for all slabs on grade, tilt walls, precast columns. Water cement ratio to be 0.50 maximum for all other concrete. Do not add water to mix at jobsite. Flyash meeting ASTM C 618 may be substituted for up to 20% of the cement content in all mixes.
- Aggregate to be per ASTM C-33.
 Water Reducing Agent (WRA). Comply with ASTM C-494.
 Air Entrainment (AE) shall comply with ASTM C-260. Provide 3-5% when
- specified.

 7. Accelerators: Dosage to be determined by contractor.
- 8. Calcium Chloride shall not be used in any concrete, for any purpose, on this project.

 REINFORCING
- All reinforcing steel to be ASTM A615, Grade 60.
 Fabricate and install all reinforcing steel according to the "Manual of Standard
- Practice for Detailing Reinforcing Concrete Structures" ACI Standard 315.

 3. Provide 2'-0" x 2'-0" corner bars to match horizontal reinforcement in poured in place walls and footings at all corners and intersections.

 4. Splices in slab on grade reinforcement shall be lapped 30 diameters or 2'-0" minimum and shall be staggered at least 4'-0" at alternate bars. All other splice locations for #6 bars or smaller, lap bars 58 diameters or 2' 0" minimum and stagger the splices at least 4'-0" at alternate bars.

 5. Provide shop drawings of all reinforced concrete items to engineer for review prior to construction of these items.
- WOOD FRAMING

 1. All lumber to be enesies and minimum grades as follows (upless otherwise noted)
- 1. All lumber to be species and minimum grades as follows (unless otherwise noted in drawings:
- 4. All bolt heads and nuts bearing on wood to be provided with a washer.

 5. All fasteners (hangers, clips, screws, nails, bolts, washers etc.) in contact with pressure treated or fire treated wood to be stainless steel or hot dipped galvanized material. Do not mix stainless steel and galvanized steel in the same connection.

 6. All nailing to be per IBC Table 2304.9.1. Obtain engineer's prior approval for all proprietary nailing or stapling systems.

 7. All nails to be common wire unless noted otherwise. Staples are not an acceptable substitute without Engineer's prior written approval. Minimum nail diameters are as

(I.E. 'HU410' hanger for 4x10 header, 'LSSU28' for 2x8 rafter, etc.).

- a. 8d = .131" b. 10d = .148"
- c. 12d = .148" d. 16d = .162"
- e. 20d = .192"

 8. Cutting and notching of joists is not permitted without engineer's prior approval. One inch diameter holes may be drilled in the center 1/3 of the member depth, but all other holes to be approved prior to drilling.

 9. Laminated beams to be Douglas Fir (Fb= 24 ksi) per AITC 117 specification. Unless noted otherwise, simple span beams to be Combination 24F-V4 and all other beams (beams cantilevered or continuous over supports, etc.) to be 24F-V8. Appearance grade to be architectural for all beams exposed to view and industrial elsewhere, unless noted otherwise in drawings. AITC or APA/EWS certificate required. Use waterproof glue.

POST-INSTALLED ANCHORS

1. All drilled expansion anchors in concrete to be "Kwik Bolt TZ" by Hilti, Inc. (ICC ESR—1917) or "Strongbolt 2 Wedge Anchor" by Simpson Strong Tie (ICC ESR—3037) only. Other expansion anchors in concrete with written approval of engineer only. All anchors to be installed following manufacturer's instructions. Provide minimum embedment, spacing, and edge distance as specified by the manufacturer for anchor size noted unless otherwise indicated on drawings. All drilled expansion anchors in concrete require special inspection during installation.

2. All drilled adhesive anchors in concrete to use "SET—XP Epoxy Adhesive" by Simpson Strong—Tie Company Inc. (ICC ESR—2508) or "HIT—HY 200 Adhesive Anchoring System" by Hilti, Inc. (ICC ESR—3187) only. Other adhesive anchors in concrete with written approval of engineer only. All anchors to be installed following manufacturer's instructions. Provide minimum embedment, spacing, and edge distance as specified by the manufacturer for anchor size noted unless otherwise indicated on drawings. All drilled adhesive anchors in concrete require special inspection during installation.

3. All Screw Anchors in concrete to be "Titen HD Screw Anchor" by Simpson Strong—Tie Company Inc. (ICC ESR—2713) or "KWIK HUS—EZ / KWIK HUS—EZ 1Carbon Steel Screw Anchors" by Hilti, Inc. (ICC ESR—3027) only. Other screw anchors in concrete with written approval of engineer only. All anchors to be installed following manufacturer's instructions. Provide minimum embedment, spacing, and edge distance as specified by the manufacturer for anchor size noted unless otherwise indicated on drawings. All screw anchors in concrete require special inspection during installation.

4. All drilled adhesive anchored reinforcement dowels in concrete to use "SET—XP"

Epoxy Adhesive" by Simpson Strong Tie (ICC ESR-2508) or the "HIT HY 200 Adhesive Anchoring System" by Hilti, Inc. (ICC ESR-3187). Other adhesive anchored reinforcement with written approval of engineer only. Install all anchors per adhesive manufacturer's instructions using ASTM A615 grade 60 dowels unless noted otherwise on plans. Provide minimum edge distance and spacing indicated by manufacturer for anchor size noted unless otherwise indicated on drawings. Provide minimum embedment noted on plans. All drilled adhesive anchored reinforcement requires special inspection during installation.

5. See drawings for anchor types required. Substituting expansion anchors for adhesive anchors, screw anchors, or cast—in anchors; adhesive anchors for expansion anchors, screw anchors, or cast—in anchors; or cast—in anchors for adhesive anchors, expansion anchors, or screw anchors is acceptable with written approval of engineer only.

6. Contractors wishing to substitute alternate anchors should submit written request, including current ICC ESR reports to engineer for approval.

Detailing, fabrication and erection of steel to conform to the Steel Construction Manual of the AISC. All steel to be A36 except as noted.

All wide flange and WT sections to be A992.
 All welds to be made with E70XX electrodes by welders certified by AWS Standards.
 Unless noted otherwise, all bolts to be A325N for steel to steel connections and A307 for anchor bolts and connections to wood. All steel to steel connections to be snug tight only. Torqueing of bolts not required unless specifically noted in detail. Provide standard plate washers under all bolt heads and nuts bearing on wood. All anchor bolts in contact with pressure treated wood to be hot dipped galvanized.
 All structural tubing to be ASTM A500 Grade B, Fy = 46 ksi. All steel pipe to be ASTM A501 (Fy = 36 ksi) or ASTM A53, Type E or S, Grade B (Fy = 35 ksi).
 All light gauge steel 54 mil and heavier shall be formed from steel with a Fy = 50 ksi. Light gauge steel 43 mil and lighter shall be formed from steel with a Fy = 33 ksi. Detail and fabricate all light gauge steel per AISI Standards. All light gauge steel sections indicated on drawings to per the Steel Stud Manufacturers Association specification. Provide 54 mil thickness material minimum at all sections which are indicated on the drawings to be welded.

8. Do not oversize drilled or punched holes with a torch.
9. All welded reinforcing noted to be ASTM grade A706. All headed stud anchors to be Nelson or approved equal. Weld all studded anchors and reinforcing noted, all around, with 1/4" fillet weld for 1/2" diameter anchors, 5/16" fillet weld for 3/4" diameter anchors, and 3/8" fillet weld for 1" diameter anchors, or alternately, use a Nelson stud welding unit.
10. Provide shop drawings of all structural steel items to engineer for review prior to fabrication.

STRUCTURAL SPECIAL INSPECTIONS

STRUCTURAL AND MISCELLANEOUS STEEL

The following special inspections are required and shall be performed by a qualified independent testing agency in compliance with the requirements of IBC Chapter 17. The testing agency shall provide copies of all test reports to the project engineer in a timely manner. Additional special inspections for non—structural elements not listed in this section are to be per the project specifications.

1. Special inspection and testing of concrete is required during the taking of test specimens and placing of all reinforced concrete per the special inspection table except slabs on grade, isolated spread footings for buildings three stories or less, continuous footings supporting light framed walls three stores or less, or concrete footings with specified f'c less than or equal to 2500 psi.

2. Special inspection is required for all structural welding and high strength bolting unless welding is performed in a shop approved by the building official. All field welding requires special inspection.

3. Special inspection is required of all post—installed anchors in concrete or masonry and

drilled anchor bolts in concrete. Inspection to be continuous during the anchor installation to insure installation meets all manufacturer's instructions and minimum embedment noted on drawings. See "POST INSTALLED ANCHORS" section of notes for more information.

4. Periodic special inspection is required of all steel stud with wood panel shear walls, holdowns, sill plate anchorages at designated shear wall locations. Periodic special inspection is required of all collectors, collector strapping and/or attachment, blocking/rim joist attachments, and wall top plate splices in shear wall lines at all locations in the building.

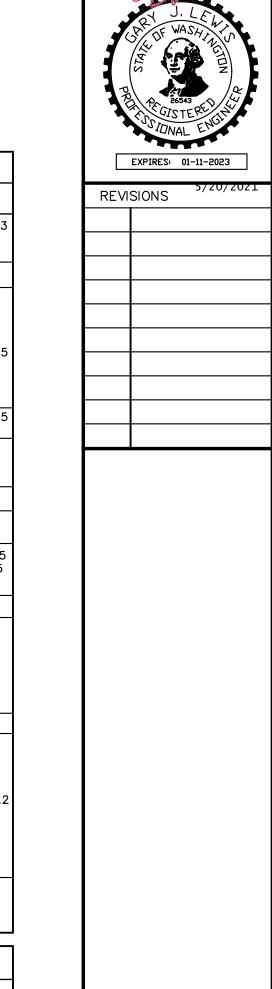
5. Periodic special inspection is required of the anchorage of emergency power systems and piping or mechanical equipment, or ductwork containing flammable or hazardous materials. The anchorage shall be in compliance with details provided by LVI or by approved details provided by the component manufacturer.

6. Periodic special inspection is required of the anchorage of suspended ceilings, access floors, and steel storage racks 8 feet or taller. The anchorage shall be in compliance with details provided by LVI or by approved details provided by the component manufacturer.

	VERIFICAT	TION AN	ID INSPECTION	CONTINUOUS	PERIODIC	VERIFY	REFERENCED STANDARD
1.	Material verification of high—strength bolts, nuts and washers:	a.	Identification markings to conform to ASTM standards specified in the approved construction documents.	-		Х	AISC 360, Section A3 and applicable ASTM material standards.
		b.	Manufacturer's certificate of compliance required	_		Х	_
	Inspection of	a.	Snug-tight joints	-	Х	-	
	high—strength bolting:	b.	Pretensioned and slip—critical joints using turn—of—nut with matchmarking, twist—off bolt or direct tension indicator methods of installation	-	Х	-	AISC 360, Section M2
		c.	Pretensioned and slip—critical joints using turn—of—nut without matchmarking or calibrated wrench methods of installation	х	-	-	
3.	Material verification of structural steel:	a.	For structural steel, Identification markings to conform to AISC 360	_	Х	_	AISC 360, Section M5
		b.	For other steel, Identification markings to conform to ASTM standards specified in the approved construction documents.	-	Х	-	Applicable ASTM material standards
		c.	Manufacturer's certified test reports	-	Х	_	_
4.	Material verification of cold—formed steel deck:	a.	Manufacturer's certified test reports	_	Х	_	_
5.	Material verification of weld filler materials:	a.	Identification markings to conform to AWS specification in the approved construction documents.	-	-	Х	AISC 360, Section A3. and applicable AWS Addocuments.
		b.	Manufacturer's certificate of compliance required.	-	_	Х	-
6.	Inspection of welding:	a.	Structural steel and cold—formed steel deck: 1) Complete and partial penetration groove welds.	х	_	_	
			 2) Multipass fillet welds. 3) Single-pass fillet welds > 5/16" 	X	_	_	AWS D1.1
			4) Plug and slot welds	X X	_	_	
			5 Single-pass fillet welds $\leq 5/16$ "	_	X	ı	
			6) Floor and roof deck welds.	-	X	_	AWS D1.3
		b.	Reinforcing steel: 1) Verification of weldability of reinforcing steel other than ASTM A706	_	_	x	
			 Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement. 	X	-	-	AWS D1.4, ACI 318: Section 3.5
			3) Shear reinforcement	x	_	_	
			4) Other reinforcing steel	-	X	_	
7.	Inspection of steel frame joint details for compliance:	a. b. c.	Details such as bracing and stiffening. Member location. Application of joint details at each connection.	_ _	X X X	<u>-</u>	_

	REQUIRED VERIFICATION AND				Т
	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1)	Inspection of reinforcing steel and placement.	-	X	ACI 318: 3.5, 7.1-7.7	1910.4
2)	Inspection of reinforcing steel welding in accordance with required verification and inspection of steel construction.	_	Х	AWS D1.4 ACE 318: 3.5.2	
3)	Inspect bolts to be installed in concrete prior to and during placement of concrete where noted on drawings.	_	X	ACI 318: 8.1.3, 21.1.8	1908.5, 1909.
4)	Inspection of anchors post—installed in hardened concrete members.	_	X	ACI 318: 8.1.3, 21.1.8	1908.5, 1909
5)	Verifying use of required design mix.	_	X	ACI 318: Ch. 4, 5.2-5.4	1904.2, 1910. 1910.3
6)	At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	Х	-	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1910.10
7)	Inspection of concrete and shotcrete placement for proper application techniques.	Х	_	ACI 318: 5.9, 5.10	1910.6, 1910. 1910.8
8)	Inspection for maintenance of specified curing temperature and techniques.	_	Х	ACI 318: 5.11-5.13	1910.9
9)	Inspection of prestressed concrete:				
	a. Application of prestressing forces.	N/A	_	ACI 318: 18.20	
	 b. Grouting of bonded prestressing tendons in the seismic force—resisting system. 	N/A	-	ACI 318: 18.18.4	
10)	Erection of precast concrete members.	_	N/A	ACI 318: Ch. 16	
11)	Verification of in—situ concrete strength, prior to stressing of tendons in post—tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	_	N/A	ACI 318: 6.2	
12)	Inspect formwork for shape, locations, and dimensions of the concrete member being formed.	_	N/A	ACI 318: 6.1.1	1910.6, 1910. 1910.8

THESE DRAWINGS AND DETAILS REPRESENT THE PROJECT ENGINEERS BEST KNOWLEDGE OF THE EXISTING BUILDING CONDITIONS. THE CONTRACTOR SHALL INVESTIGATE THE EXISTING CONDITIONS IN THE FIELD AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES.



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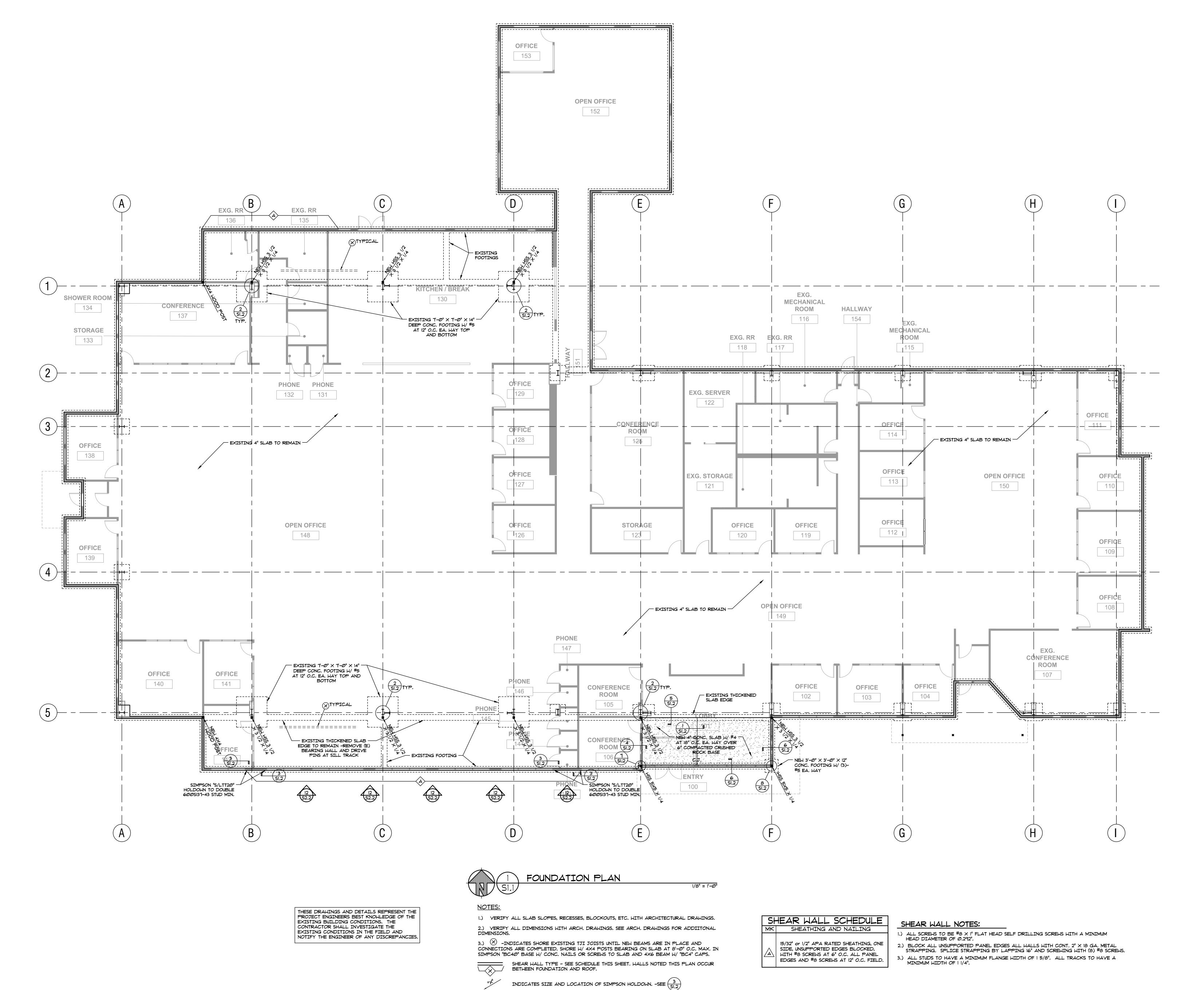
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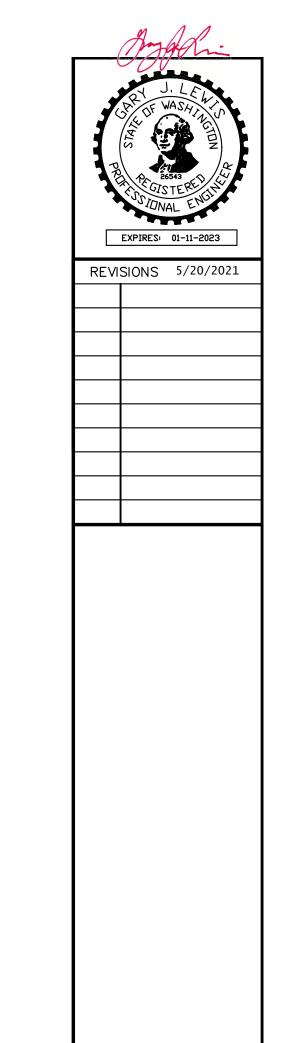
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USNR REMODEL

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MOODLAND, MA 98674

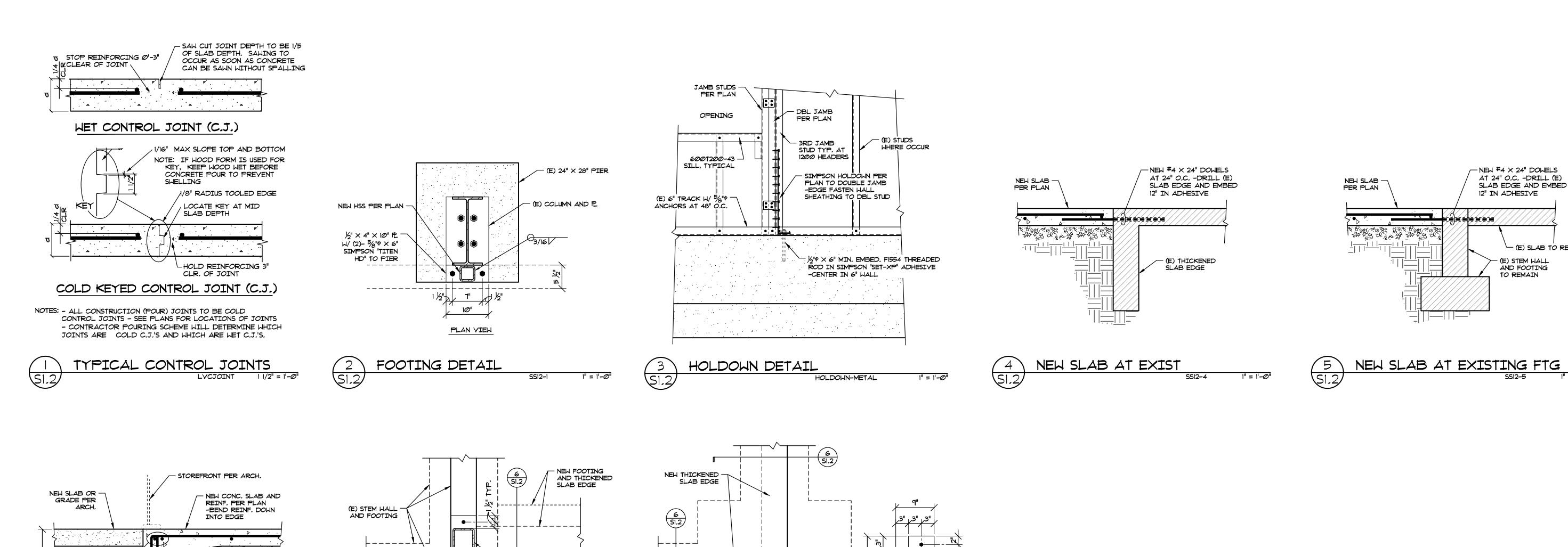
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HSS COLUMN —

L______

─ BASE 党 ½" × 6" × 11" W/ (2)- ── ½" Ф × 6" EMBED. F1554

THREADED RODS IN SIMPSON
"SET-XP" ADHESIVE

FOOTING PER PLAN

PLAN SECT. AT COL. \$ FTG.



EXPIRES: 01-11-2023

REVISIONS

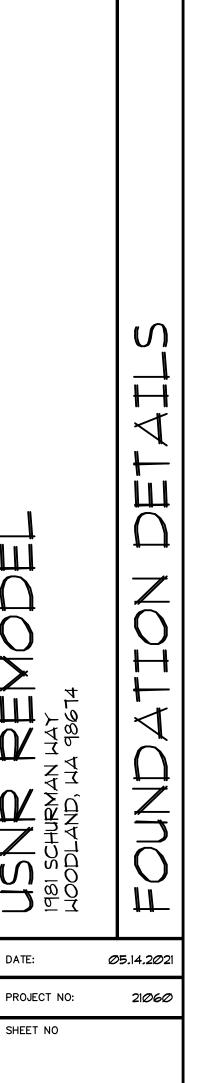
NEW #4 × 24" DOWELS AT 24" O.C. -DRILL (E) SLAB EDGE AND EMBED

(E) STEM WALL

AND FOOTING TO REMAIN

(E) SLAB TO REMAIN

12" IN ADHESIVE



- RADIUS SUBGRADE

— CONC. FOOTING -SEE PLAN FOR COLUMN FOOTING

H----

HSS COLUMN —

PER PLAN

─ BASE 电 ½" × 6" × 11" W/ (2)-

"SET-XP" ADHESIVE

PLAN SECT. AT COL. \$ FTG.

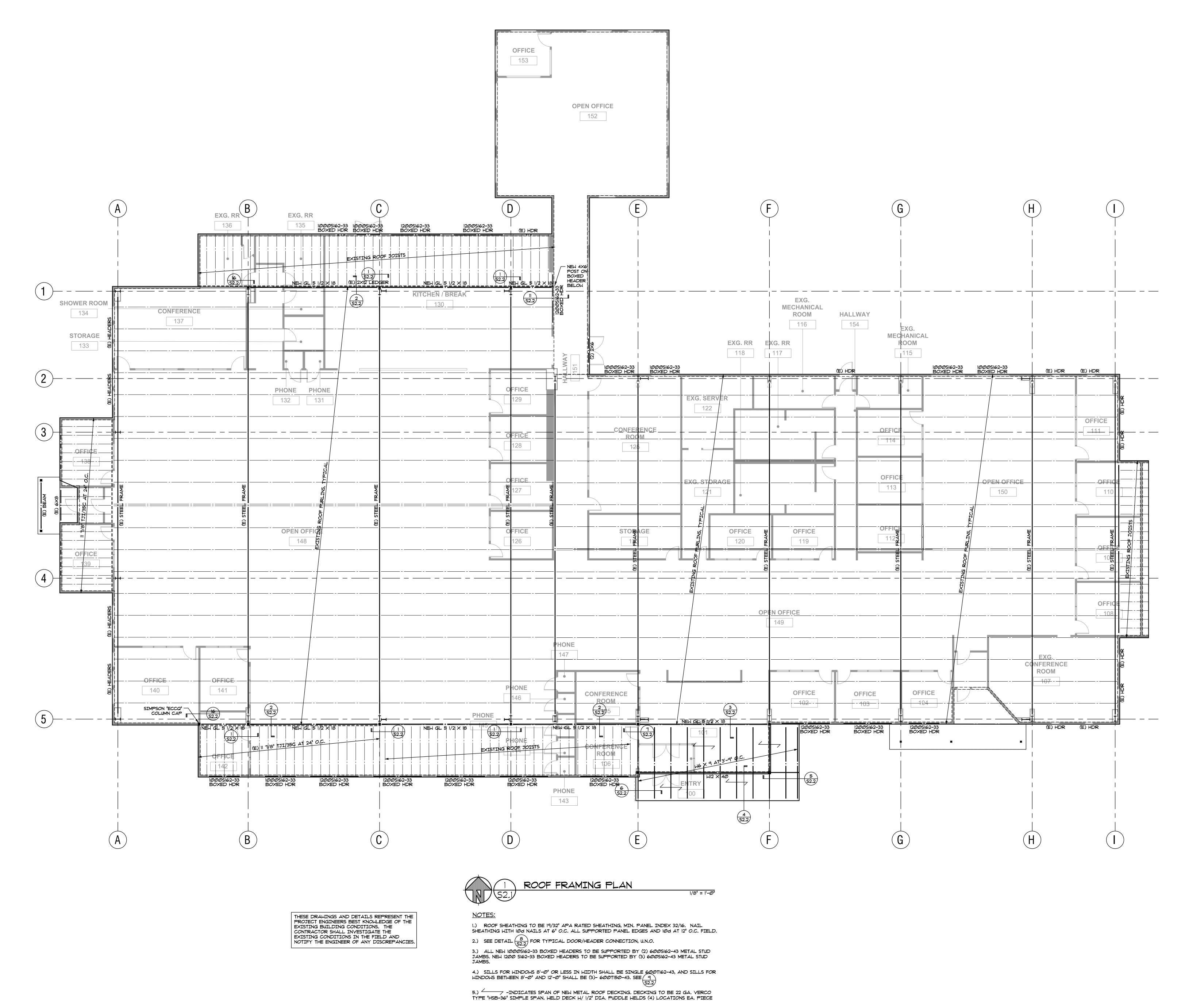
½"Φ × 6" EMBED. F1554 THREADED RODS IN SIMPSON

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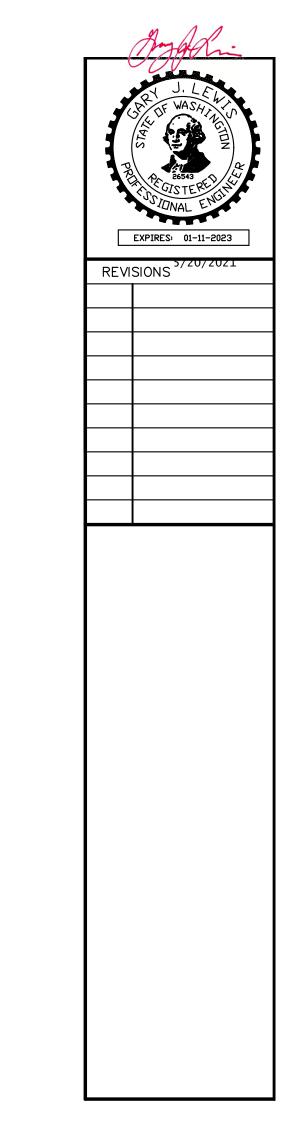
(2)- #4 CONT.-

SLAB EDGE AT ENTRY
SSI2-

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EA. SUPPORT AND AT 18" O.C. AT SUPPORTS PARALLEL TO DECK. BUTTON PUNCH SEAMS AT 24" O.C.





USNR REXOUELL

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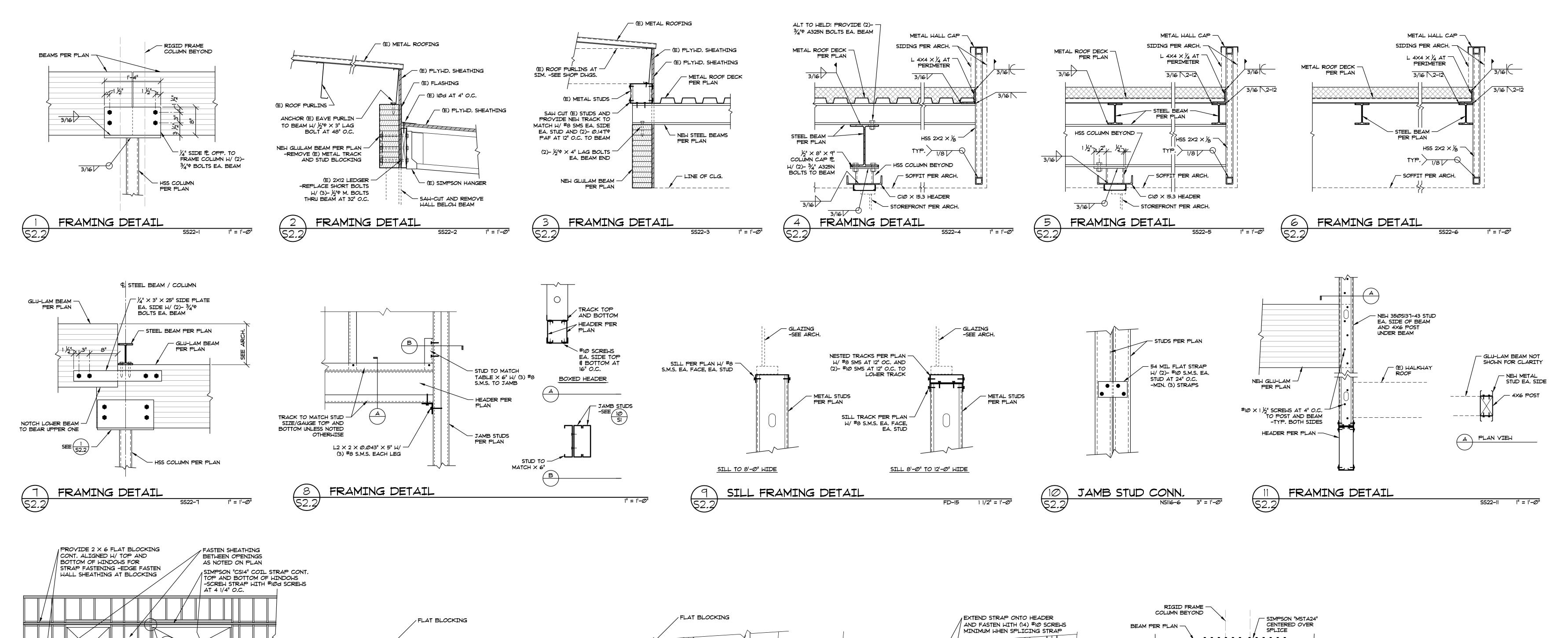
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DATE: Ø5.14.2021
PROJECT NO: 21060

SHEET NO



HEADER PER PLAN

JAMB STUDS PER PLAN

SHEARSTRAP-MTL2

\SIMPSON "CSI4" STRAP

SHEAR WALL ELEVATION

|-----|| |------||

|||||||

HEADER PER PLAN

SHEARSTRAP-MTL3

¼" SIDE PL OPP. TO -

¾"¢ BOLTS

HSS COLUMN -

PER PLAN

FRAMING DETAIL

FRAME COLUMN W/ (2)-

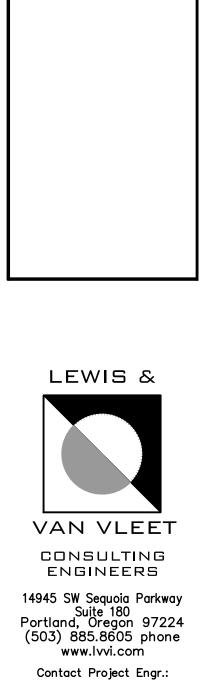
SHEAR WALL ELEVATION

CONT. SILL PLATE PER

\SIMPSON "CSI6" STRAP

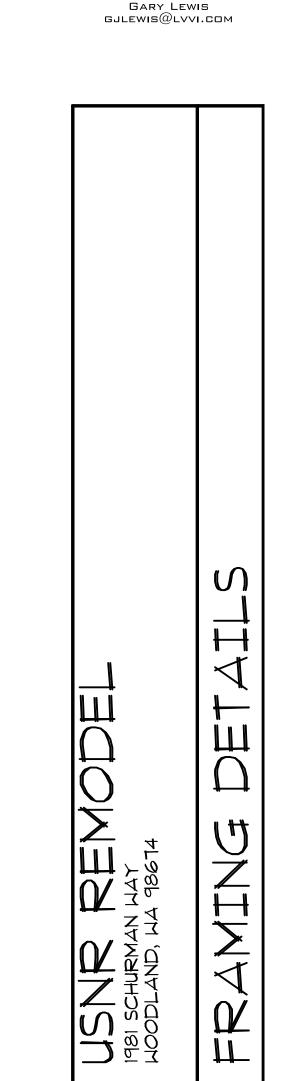
PLAN -NO SPLICES

SHEAR WALL ELEVATION



EXPIRES: 01-11-2023

REVISIONS



SHEET NO

(NO SPLICES) TYPICAL

2.) SPLICE ALL 'CS' STRAPS AT HEADERS ONLY. FASTEN EACH STRAP TO HEADER WITH (14) #10 SCREWS MINIMUM. SEE 15

3.) LOCATE 'C' STRAP ON OUTSIDE FACE OF WALL AND RUN FULL LENGTH ABOVE AND BELOW OPENINGS UNLESS NOTED OTHERWISE ON PLAN.

ELEYATION-MTL

1.) SHEAR WALL TYPE INDICATED ON PLAN NOTES ATTACHMENT

BETWEEN OPENINGS AS INDICATED ON PLAN.

SHEAR WALL DETAIL